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Implementation of Structural Eurocodes in Malaysia



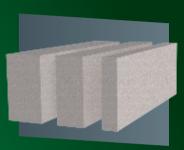






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IEM DIARY OF EVENTS

One Day Symposium on Protection Against Lightning

27th November 2014

Organised by	: Electrical Engineering Technical Division
Time	: 9.00 a.m. – 5.00 p.m.
Venue	: Grand Dorsett, Subang, Malaysia
CPD/PDP:	: 6.0

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.



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"Implementation of Structural Eurocodes in Malaysia"

by **Ir. Hooi Wing Chuen** Chairman, Civil and Structural Technical Division, IEM

CIVIL & Structural Engineering is possibly the oldest form of engineering known to man. Over thousands of years it has evolved in tandem with the needs of its communities. However, standards of development of communities around the world are naturally quite different, making growth across borders difficult.

On the other hand thanks to a common standards platform, the financial, information and communication technology industries have become virtually borderless today. Similarly, the Civil & Structural Engineering industry should pursue a set of standards common to most if not all global communities. When the British Standards Institution announced its plans to withdraw the British Standards, the IEM C&STD took the opportunity to survey and study the many standard codes of practices available around the world. The conclusion was that the EN Standards (Eurocodes) would best fit Malaysia's needs.

The IEM C&STD has since embarked on a long term project to equip our nation's Civil & Structural Engineers with, in our opinion, the most dynamic and up to date codes of practice available. The IEM C&STD has spearheaded the adoption of the Eurocodes as Malaysian Standards. The result of many months of work is the publication of the latest series of MS EN standards together with the very important National Annexes.

IEM is playing its part in spreading awareness and knowledge of the new MS EN standards and their implementation. In doing so, the IEM has organised road shows, courses, talks, published articles and engaged Government Departments on the new standards. To date, we have successfully produced 5 sets of MS EN standards and are seeing a good number of Civil & Structural engineers including the JKR apply the new standards.

In this issue of the Jurutera, we showcase the implementation plan for these standards and some more of the work that had been done. I would like to thank all the people who had led and contributed to the realisation of these MS EN standards and at the same time would like to take this opportunity to call on our population of over 2,700 registered C&S engineers/members to get involved in this initiative and to volunteer to work in our standards committees or work groups. With a further 53 ENs to review, there are lots more work to do and much knowledge to be gained.

Carpe Diem. Seize the day and send an email to the IEM Secretariat to and state your interest.

Thank you. 🔳

Ir. Hooi Wing Chuen is currently the Vice President Engineering and Planning for Sara-Timur Sdn. Bhd. He has over 25 years experience in the Cement, Concrete and Construction industries in New Zealand, Hong Kong, Macau, Zhuhai (PRC) and Malaysia. His expertise includes design, engineering, manufacture & construction of precast concrete structures and pavements.



COVER NOTE

COVER STORY

Towards the Implementation of Structural Eurocodes in Malaysia

by A. Pfordten

THE CURRENT SITUATION

As a former British colony, Malaysia had been using British Standard in the construction industry in the past. However, the United Kingdom has announced that it would no longer update the British Standard as it would be using a new standard known as Eurocode.

Ir. Prof. Dr Jeffrey Chiang, who is part of the IEM technical committee charged with developing the

Eurocode, explains why Malaysia needs to adopt the Eurocode.

"Since the British Standard has been withdrawn, there would no longer be further updates or improvements. If we continue to use British

⁶⁶If we continue to use British Standard, we would be stuck in terms of advancement.⁹⁹

Ir. Dr Jeffrey Chiang

Standard, we would be stuck in terms of advancement," he said.

Through its Civil and Structural Engineering Technical Division, IEM has to set up a Position Paper Committee in 2002.

"We invited all the various stakeholders from the industry, academia and government, private sector and so on, to come in together to discuss the options available once the British Standard is withdrawn in 2010. The standards are important because the practising engineers need to design according to certain standards so that the documents can be submitted for approval by the authorities."

The authorities involved are local councils such as Dewan Bandaraya Kuala Lumpur (DBKL) and Majlis Perbandaran Petaling Jaya (MPPJ).

"The British Standard was accepted because it had been stated or provided for in the Uniform Building By-Laws (UBBL) in Malaysia as outlined by the Ministry of Housing and Local Government, which is the custodian of the building by-laws," said Ir. Dr Chiang.

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A key improvement in the new standard is that Eurocode allows for a National Annexe which is, essentially, another document to supplement the main code or main standard.⁹⁹

Ir. Dr Jeffrey Chiang



by Ir. Prof. Dr Jeffrey Chiang

by Ir. Tu Yong Eng

Currently these laws state that the British Standard is the accepted standard for use in Malaysia. If there are any changes in the standard used, then it will conflict with the by-laws.

Ir. Dr Chiang said IEM had studied the issue and came to the conclusion that the Eurocode was the way to go.

He said: "We have come to the conclusion that it is

logical and reasonable to follow the steps taken by UK. In other words, we will adopt the Eurocode, in line with the withdrawal of the BS in Malaysia. So now, the problem is this: When is the deadline to do this?"

In the UK, the BS was

officially withdrawn and the Eurocode fully adopted in March 2010.

"Prior to that, there was a two-year transition period during which engineers could choose to use either the BS or Eurocode in their work," he said. That transition period allowed local engineers to familiarise themselves with and learn to use the Eurocode. After the two-year transition period, they would drop the BS totally. Currently, they are fully compliant with Eurocode.

"Similarly, Singapore, a Commonwealth country, faced the same dilemma but they were quicker in making a decision. In April 2013, they decided to start the twoyear transition period. By May next year, Singapore will fully comply with the Eurocode standards and BS will be withdrawn," said Ir. Dr Chiang. "In Malaysia, we are facing

> a situation where we haven't come to a decision yet. The Ministry of Housing and Local Government has yet to make a decision. We were told in late 2012 that the UBBL was being revised after receiving input from all the professional bodies including IEM. We haven't seen the actual revision yet but we are told that the Eurocode is now included or has replaced the BS in the revised UBBL 2012 version.

"But there is a snag. The Ministry cannot enforce the adoption of Eurocode in line with UBBL because it has to be enforced by the local authorities. They are the ones

which gazette, implement and enforce and they haven't done this yet. We were informed that the Selangor State Secretariat had already accepted or gazetted the UBBL revised version. However, it has to be accepted by all the other local authorities and not just the Selangor State Secretariat," he said.

⁶⁶ IEM also is keen to organise seminar courses, awareness talks and dialogues as part of the partnership programme with Standards Malaysia.⁹⁹

Ir. Dr Jeffrey Chiang

"A dilemma has surfaced. When we approached MPPJ, it said it was not aware of the changes. It had yet to come to a decision. In other words, it cannot be enforced for now. Our own professional members here have also come to a seek advice from the Institution."

MATTER OF NAME

Ir. Dr Chiang said IEM had been working with Standards Malaysia ever since the presentation of the IEM Position Paper in early 2004.

"Up to now, we have already worked together with Standards Malaysia to produce the MS standards in line with Eurocode. They named it as MS EN. It's a Malaysian Standard but adopts Eurocode as the basis. It's the same as Singapore, which also uses the same sort of name – SS EN. In the UK it is BS EN. That's how they name it. Different names are used in different countries, but essentially it is the same Eurocode," he said.

Ir. Tu Yong Eng, another member of the committee, said the key difference was whether it was mandatory or not.

"Rightfully, in Malaysia, we can use it. In the current practice, we can also use the American standard and the Australian standard and so on," he said.

Ir. Tu said that for a country to adopt the Eurocode, they must first publish a National Annexe.

"Eurocode usually consists of three parts. The main text will be the same for all countries adopting Eurocode. This

means that for Germany, Denmark, UK, Malaysia and Singapore, the first part will be the same document. The second part varies from country to country. It is called the National Annexe. Then, the other third document is called the Non-contradictory, Complementary Information or NCCI document," he said.

⁶⁶ The Eurocode is a state of the art international standard which many countries in the region, in particular Commonwealth countries, will adopt. ⁹⁹

Ir. Dr Jeffrey Chiang

Ir. Tu added that every country could publish the NCCI documents, on condition that it is not in conflict with the main text. It only serves as a complementary document to

the main text.

"When we adopted the British Standard, we adopted it in full in most cases. But for the Eurocode, we have to publish our own National Annexe. This is the main difference compared to the previous BS. We have set up a lot of technical committees, not only to adopt draft the Eurocode, but

also to serve as the training ground for trainers. This means we should get a group of experts who are well versed in the document. In a way, we do not want to adopt a standard where Malaysia may not have the supporting technical documents and experts.

LOCAL CONTENT THROUGH THE USE OF NATIONAL ANNEXES

A key improvement in the new standard is that Eurocode allows for a National Annexe which is, essentially, another document to supplement the main code or main standard.

"The National Annexe allows local parameters to be included. For example, variables like local weather condition, local practices and materials which are indigenous to a country. All these are local conditions or parameters which we can put into the National Annexe to go with the Eurocode. In practice, countries can adopt the Eurocode, but they can still adopt local conditions as well," said Ir. Dr Chiang.

In that sense, he added, IEM has already drafted and produced the MS EN documents with Standards Malaysia, which comprise a total of 10 documents encompassing concrete design, steel design and geotechnical design.

"It is still not completed. We have just finished the first parts of five Eurocodes. However, each one Eurocode has multiple chapters. To be frank, it will take many years to complete. The first parts are already available through

> Standards Malaysia. It is the owner of this standard. IEM is working on its behalf. It is the one that will sell or distribute to the users," he said.

While many of the documents are already available, Ir. Dr Chiang said there is still a lot of work to be done. "It's still ongoing...

the work of producing the necessary MS EN

COVER STORY

documents. The work has never stopped. Standards Malaysia is also asking us to promote it. This is why IEM is holding a lot of seminars and courses as well as awareness talks and so on, to publicise it to our members and other stakeholders in the industry. This is what we are doing now with Standard Malaysia in a partnership arrangement," he said.

PROGRESS ACHIEVED BY IEM TECHNICAL COMMITTEES

According to Ir. Dr Chiang, there are currently a total of 58 documents.

"There are a total of 10 parts. Each topic has sub parts. If you count all sub parts together, then you have a total of 58 documents. It takes time to publish the National Annexe. Of course you may not publish all, as some parts may not be relevant or important, for example, structural aluminum which not many people actually use," he said.

The codes that have currently been documented are as follows:

- MS EN 1990 Basis of Structural Design the key code
- MS EN 1991-1-1 Actions (Loading)
- MS EN 1992-1-1 Concrete Structures Design
- MS EN 1993-1-1 Steel Structures Design
- MS EN 1997-1-1 Geotechnical Engineering

These are the five codes already published as MS EN. Each has an accompanying National Annex.

The IEM technical committee is also working on the Eurocode 8 or MS EN 1998-1-1 for earthquake design and MS EN 1991-1-4 on

In order to be competitive in the region and globally, the Eurocode must be adopted.⁹⁹

Ir. Dr Jeffrey Chiang

wind loading. This process is still ongoing. IEM will form technical committees to address other parts of the concrete and steel design which have not yet been completed.

THREE-YEAR TRANSITION PERIOD

In April 2014, the three main engineering bodies in Malaysia – IEM, the Board of Engineers Malaysia (BEM) and the Association of Consulting Engineers Malaysia (ACEM) – met and drafted a letter which was addressed to the Ministry of Housing and Local Government, seeking its agreement to commence a transition period, like what was done in the UK and Singapore.

"In that letter, we stated that the transition period should start from 1st June this year. Instead of two years, we wanted a three-year period starting 1st June, 2014 and ending in May, 2017, to give us more time," said Ir. Dr Chiang.

He felt that three years should be long enough for the local authorities to push enforcement, for local engineers to learn up on the code and for universities to teach undergraduate students using the Eurocode as the basis for standards.

It would also allow IT software companies to upgrade software to include Eurocode and for the local authorities to be prepared.

"After all, local authorities are ones which have to approve all submissions by practising engineers for the approval of building design and so on. So they should be well versed in Eurocode," stressed Ir. Dr Chiang who added that the three-year period should be long enough for the authorities to familiarise themselves with this knowledge.

IEM is currently working closely with Standards Malaysia to contact and coordinate with government agencies such as the Public Works Department, the Construction Industry Development Board (CIDB), DBKL and various other engineering bodies as well as interested parties.

It also is keen to organise seminar courses, awareness talks and dialogues as part of the partnership programme with Standards Malaysia, which is keen to promote the use of the Malaysian standard. That is their objective. IEM wants to get local engineers to upgrade themselves, said Ir. Dr Chiang.

"The Eurocode is a state of the art international standard which many countries in the region, in particular Commonwealth countries, will adopt. Countries like Singapore, South Africa, India and Hong Kong will be

> using this code because the British Standard will no longer exist. So there is no point in sticking to the old standard," he said.

THE MAIN CHALLENGES

The biggest challenge is

changing the mindset, said Ir. Dr Chiang. "The people here are slow to adapt to change. The word 'change' itself gives them the impression that they would need to do a lot more work to adopt a new code or a new standard. The Eurocode is not easy. The document itself is thick, and there are many provisions in it which require a lot of changes, in terms of philosophy," he said.

"It is very different from the British Standard although the fundamentals are basically still the same. People, especially those in the middle management levels and higher, may not want to learn up the code because, as you know, they may be a bit rusty. In any case, they don't do the actual design work anymore. They have already moved on to management. It's the upcoming generation of engineers who will soon take over the industry.

"This is why we will allow them more time to catch up. Perhaps by next year, if the universities take up this issue of Eurocode, they will churn out graduates who are fully trained in Eurocode. Then slowly, the majority of engineers in Malaysia will be Eurocode trained," he said.



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COVER STORY

Ir. Dr Chiang added that even if senior engineers are reluctant to learn the Eurocode, they should encourage the younger generation do so. "Just accept the change. Don't deny it, obstruct or insist that young engineers must still use the British Standard. There is no point in that," he said. In order to be competitive in the region and globally, the Eurocode must be adopted.

We are not just competing with Singapore but also with up-and-coming countries like Vietnam, Cambodia and Thailand.99

Ir. Dr Jeffrey Chiang

"We are not just competing with Singapore but also with up-and-coming countries like Vietnam, Cambodia and Thailand. All these countries are becoming more competitive in terms of cross-border engineering projects. Once our doors are open, there will be an influx of engineers from the region coming in. They will take over if we are not capable enough in terms of technology, knowledge and expertise," said Ir. Dr Chiang.

"We are also exporting our services overseas. We have engineers working in Dubai, Arab and Africa. If they don't use Eurocode, how can they compete against other countries? Will British or UK companies employ them? These are the challenges we have to face because the world is moving fast."

Ir. Tu pointed out that another big challenge lies in the cost or budget for retraining. A lot of money would be required to do this and there would be other costs as well.

"For the industry to adopt the changes, the regulatory industry has to be upgraded in terms of the laboratory and testing methods. There will be some changes to the manufacturing process as well. These challenges are not easy to overcome."



Ir. Prof. Dr Jeffrey Chiang Choong Luin is currently the Dean of the Faculty of Engineering & Built Environment at SEGi University. His research interests are in structural behaviour of concrete structures in shear, wind loadings and earthquake design for structures.

He is an alumni of the Wollongong University NSW Australia. He graduated with a BEng (Hons) Civil, in 1991 and obtained his PhD in 1995. Before pursuing a career in the academia, he worked as a structural engineer with Arup Jururunding Sdn Bhd KL. He played a key role in drafting the Malaysian Standards on design of concrete structures (as TC Secretary). He is now serving as Chairman of IEM Technical Committee on Earthquake, and formerly chaired the IEM Technical Committees on Wind Loading

Previously, he was Chairman of the IEM Civil & Structural Engineering Technical Division, and served as the IEM Honorary Secretary and Treasurer. Recently he was elected again as Honorary Treasurer of IEM. He was also the former WG coordinator on design of the International Committee on Concrete Model Code for Asia.

Ir. Tu Yong Eng is a professional engineer and amateur mathematician. He has extensive experience in civil and structural, design and construction. He has been continually involved in education and research, participating and organising various courses, seminars and conference.

He was a member of various technical committees which drafted several Malaysian Standards. He was also an advisor to several local colleges and universities.

Ir. Tu held several key positions in The Institution of Engineers Malaysia. He is a Council member of the IEM (2008-2011, 2012-2015), Advisor for Civil and Structure Engineering Technical Division (2009-2011), Committee member, 9th (2006) 10th (2009) and 11th (2012) International Conference on Concrete Engineering And Technology, Secretary, IEM Position paper on the Prevention of Collapse of Scaffolding and Temporary Works. He has published more than 10 papers in both local and international bulletins and conferences.





Implications and Timeline to Implement Eurocodes in Malaysia



by Ir. Prof. Dr Jeffrey Chiang

1.0 INTRODUCTION

In May 2010, UK withdrew British Standards relating to structural design and implemented Eurocodes. Singapore followed suit, but it also introduced a transition period of two years (from April 2013), during which period both British Standards and Eurocodes would co-exist for application and submission for approval purposes by the local authority.

So what are Malaysia's plans to introduce and implement the adoption of Eurocodes in the construction industry here? How can The Institution of Engineers Malaysia (IEM) play a role in providing the incentives and support to ensure that the transition from BS codes to Eurocodes is carried out in a smooth and systematic manner?

2.0 SIGNIFICANCE OF THE ISSUE AT HAND – IMPLEMENTATION OF EUROCODES IN MALAYSIA

At the moment, the decision-makers are not aware of the seriousness in the switchover of design standards for structural design from BS to Eurocodes. It all boils down to the provisions in the Uniform Building By-Laws (UBBL) in which it currently still stipulates the use of British Standards as the de facto approved standards for submission purposes.

The custodian of the UBBL is the Ministry of Housing & Local Government, while the Local Authority is the body that implements the policies or provisions therein.

Back in 2002, IEM mooted the formation of the IEM Position Paper Committee to draft a Position Paper titled "Design Standards for Concrete Structures in Local Construction Industry After 2006".

At that time, UK had given an undertaking that it will introduce Eurocodes as the structural design standards to replace the British Standards in use at that time – such as BS 8110 (for concrete structures) and BS 5950 (for steel structures), to name two.

Since then, IEM has produced the following MS EN 199x documents based on Eurocodes, which are published by Standards Malaysia or the Department of Standards Malaysia (DSM):

- MS EN1990:2010 Eurocode: Basis of Structural Design (RM120)
- NA to MS EN1990:2010 Malaysia National Annex to Eurocode: Basis of Structural Design (*RM20*)
- MS EN1991-1-1:2010 Eurocode 1: Actions on Structures Part 1-1: General Actions – Densities, Self-weight, Imposed Loads for Buildings (*RM50*)
- NA to MS EN1991-1-1:2010 Malaysia National Annex to Eurocode 1: Actions on Structures – Part 1-1: General Actions – Densities, Self-weight, Imposed Loads for Buildings (*RM20*)
- MS EN1992-1-1:2010 Eurocode 2: Design of Concrete Structures Part 1-1: General Rules and Rules for Building Structures (*RM230*)
- NA to MS EN1992-1-1:2010 Eurocode 2: Malaysia National Annex to Design of Concrete Structures – Part 1-1: General Rules and Rules for Building Structures (*RM30*)
- MS EN1993-1-1:2010 Eurocode 3: Design of Steel Structures Part 1-1: General Rules and Rules for Building Structures (*RM100*)
- NA to MS EN1993-1-1:2010 Eurocode 3: Malaysia National Annex to Design of Steel Structures – Part 1-1: General Rules and Rules for Building Structures (*RM20*)
- MS EN1997-1:2012 Eurocode 7: Geotechnical Design Part 1: General Rules (*RM170*)
- NA to MS EN1997-1:2012 Eurocode 7: Malaysia National Annex to Geotechnical Design – Part 1: General Rules (*RM30*)

These are available for purchase online or at Department of Standards Malaysia's headquarters in Cyberjaya.



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3.0 THE KEY DOCUMENT – THE UNIFORM BUILDING BY LAWS (UBBL)

It must be highlighted that construction engineering practices in Malaysia are largely governed by the provisions in the Uniform Building By Laws (UBBL) – which has its latest amendments being approved in 2012. And the latest amendments in the UBBL 2012 include the replacement of British Standards by Eurocodes being adopted as Malaysian Standards, named as MS EN standards, particularly for documents already prepared for concrete and steel design and others, as stipulated above.

Therefore, in order for the construction industry and related professional engineering practices to move forward, it is imperative that Eurocodes be used eventually as the referenced design standards for submission purposes.

However, not all local authorities have started to implement the revised UBBL, even though Selangor State has officially adopted the revised UBBL. This has yet to be filtered down to the level of the local authorities.

4.0 TIMELINE FOR IMPLEMENTATION

The three main engineering bodies in the country – Board of Engineers Malaysia (BEM), The Institution of Engineers Malaysia (IEM) and Association of Consulting Engineers Malaysia (ACEM) – came together and drafted a letter dated 5th June 2014, addressed to the Ministry of Housing and Local Government (Kementerian Perumahan dan Kerajaan Tempatan, KPKT) to inform the Ministry of the Joint Stand taken by them with regards to the adoption and implementation of MS EN Eurocodes in Malaysia.

The three had come to the consensus that a proposed 3-year transitional period (or moratorium period) would be a good way to tackle the implementation of Eurocodes, as it would allow sufficient time for local practitioners to be familiar with the new standards. This was in line with other countries which had adopted similar measures. For example, UK implemented a 2-year transition period (2008 to 2010) and Singapore has put in place a 2-year transition period (April 2013 to May 2015).

In the letter to KPKT, it was suggested that the 3-year transition period would start from 1st June 2014 and end on 31st May 2017. This would enable adequate preparation and training of local engineers and university engineering students on the use of Eurocodes in design courses, so that they would be ready by 2017.

Even the Ministry of Education can come out with a directive to all local public and private universities to conduct all design structural courses with references to Eurocodes, starting from, say, 2015. A 2-year period should be enough to fully implement Eurocodes into the teaching of relevant courses at university level. The teaching of design courses normally commences from Year 3 for a typical 4-year engineering programme. A 3-year UK-degree course would have design courses taught from Year 2. This is when the use of MS EN Eurocode 2 will have to be used in the teaching of design of concrete structures, in place of the withdrawn British Standards BS8110.

The use of Eurocodes will be more prevalent when students go on to take their Final Year Projects where emphasis will be on the use of design standards, and in integrated engineering design, where multi-disciplines are involved in the design of a typical project given to groups of students, most likely in Year 3.

Besides the education field, the IT software industry will also require time to institute and to fine-tune the inclusion of Eurocodes as a design suite or package into commercial software. Currently, most if not all software packages in the market, have in place Eurocodes as a choice of design standard for users, in order to process their analysis and design steps on using their commercial products.

Last but not least is the mindset of engineering graduates. Eurocodes are here to stay and, if graduates want to be competitive both locally and on the international arena, they definitely have to be well-versed in the adoption and application of Eurocodes.

Figure 1 shows a representation of a tentative timeline and milestone for Malaysia, in adopting fully Eurocode as design standards in place of British Standards by 2017.

Jan-14	Feb-14	Mar-14	Apr-14 Consensus between BEM / IEM / ACEM	May-54	Jun-14 Start 3-yr transition EC2 / BS8110	36.14	Aug-14 Push for adoption UBBL by Local Auth	Assarsmass seminars on EC2	Official directive from	Nor-14 Official desctive from MOE	Dec-14
Jan-15 Start premotional events by DSM	Feb-15		Anareness seminars on EC2	May-15	Jun-15 Start trial acceptanc EC2 as submissio	Avantesi seminara on EC2		Sep-15 Intertative courses or EC2 by IEM		Nov-15	Dec-15
Jan 16 Manses on EC2 by EM	Feb-16		Apr-16	May-16	Jun-19 DSM to review EC2 warge in industry		Aug-16 Intensive Courses or EC2 by EC3	Sep 16	Out-15	Nov 16	Dec-16
Jan 17	Feb-17	Mar-17 Intensive Courses of ECZ by ECZ by ESM	Age-17	May-17		Jul-17 Start ME adoption of EC2	Aug 17	Sep 17	Oct-17 Intensive Courses or EC2 by IEM	Nev 17	Dec-17

Figure 1: Proposed timeframe for 3-year transition period towards full adoption of Eurocode by 2017

5.0 TRAINING PROGRAMMES AS PART OF ACTION PLANS

Since 2009, IEM, through the Civil & Structural Engineering Technical Division, has been actively organising seminars and courses on Eurocodes, especially Eurocode EC2 (Concrete Design) as well as courses and workshops on Eurocode EC8 (Design for Earthquake Resistance).

Some of the more recent training seminars and courses offered or conducted by IEM on Eurocode 2 are:

- 17-18 October 2013, 2-Day Course on "Simplified Unified Practical Design to MS EC0, MS EC1 & MS EC2 From a Consulting Engineer's Perspective", by Ir MC Hee at Armada Hotel, PJ. (95 participants)
- 11 June 2012, 1-Day Pre CONCET 2012 Conference on "Design of Concrete Structures to EN 1992", by Dr WMC McKenzie, PICC, Putrajaya. (61 participants)
- 21 May 2010, 1-Day Seminar on "Adapting Eurocodes EC0, EC1 and EC2 in Malaysia" by various presenters, Dynasty Hotel, Miri, Sarawak. (36 participants)
- 10-12 March 2010. 3-Day Course on "Practical Design to MS EC0, MS EC1, MS EC2 from a Consulting Engineer's Perspective", by Ir MC Hee at Armada Hotel, PJ. (71 participants)
- 12 November 2009, 1-Day Seminar on "Adopting Eurocodes EC0, EC1 and EC2 in Malaysia" by various presenters at Puteri Pacific Hotel, Johor Baru (73 participants).
- 29 October 2009, 1-Day Seminar on "Adopting Eurocodes EC0, EC1 and EC2 in Malaysia" by various presenters at Evergreen Laurel Hotel, Penang (101 participants).
- 14 October 2009, 1-Day Seminar on "Adopting Eurocodes EC0, EC1 and EC2 in Malaysia" by various presenters at Armada Hotel, PJ (161 participants).
- 20 June 2009, Half-day Awareness Seminar on "Eurocodes -EC0, EC1 and EC2" by Ir MC Hee at UTAR Campus, PJ (101 participants).

It is IEM's policy to charge relatively low attendance fees for courses that it organises, so as to enable maximum participation by stakeholders in the industry but priority will always be given to its members, whether they are students, graduates or corporate members.

Other than holding such public activities to generate publicity

and knowledge on the use of Eurocodes, the IEM Library also has many volumes of up-to-date references and even software on the use and adoption of Eurocodes.

The only thing that is lacking is the availability of the Malaysian Standards MS EN publications, since Standards Malaysia does not appoint NGOs like IEM to be an agent in the sales of MS publications on IEM premises. Those who wish to purchase such publications have to contact Standards Malaysia or SIRIM.

IEM is in the midst of planning and organising a series of awareness seminar campaigns this year, in both the peninsula and Sabah and Sarawak. This is made possible with the SMART Partnership Programme between IEM and Standards Malaysia, in which the latter provided a sum of RM100,000 as a grant to promote the use of MS EN publications. IEM will also consider the possibility of bringing some MS EN documents (with the permission of Standards Malaysia) for purchase by interested participants.

6.0 WHAT MS EN STANDARD DOCUMENTS ENTAIL

The following are brief descriptions of the developed and published MS EN documents which are essential for the adoption of main parts of Eurocodes for the local construction industry, principally on concrete and steel design as well as geotechnical design.

6.1 MS EN1990:2010 – EUROCODE: BASIS OF STRUCTURAL DESIGN

This is the must-read first document because it sets the basis of structural design for all Eurocodes that follow from concrete design to steel design all the way to aluminium and timber design. It is the head key code for the harmonised Structural Eurocodes. MS EN 1990 establishes for all the Structural Eurocodes the principles and requirements for safety and serviceability and provides the basis and general principles for the structural design and verification of buildings and civil engineering structures (including bridges, towers and masts, silos and tanks, etc.). MS EN 1990 gives guidelines for related aspects of structural reliability, durability and quality control. It is based on the limit state concept and is used in conjunction with the partial factor method.

6.2 NA TO MS EN1990:2010 – MALAYSIA NATIONAL ANNEX TO EUROCODE: BASIS OF STRUCTURAL DESIGN

This has to be read together with the main document, MS EN1990, because it highlights the provisions which allow for local parameters which do not conform exactly to conditions elsewhere. Where required, local design parameters are recommended, otherwise those recommended by BSI will be adopted.

6.3 MS EN1991-1-1:2010 – EUROCODE 1: ACTIONS ON STRUCTURES – PART 1-1: GENERAL ACTIONS – DENSITIES, SELF-WEIGHT, IMPOSED LOADS FOR BUILDINGS

MS EN 1991-1-1 covers the assessment of actions for use in structural design due to:

- · the density of construction materials and stored materials
- the self-weight of structural elements and whole structures, and some fixed non-structural items and
- imposed loads on floors and roofs of buildings (but excluding snow, which is covered by BS EN 1991-1-3, *Snow loads*).

The scope of MS EN 1991-1-1 is greater than for the appropriate BS codes (BS 6399-1 and BS 648). There remain some topics

(e.g. vertical loads on parapets and values for actions for storage and industrial use) that are not covered as comprehensively in MS EN 1991-1-1 when compared to BS 6399.

FEATURE

6.4 NA TO MS EN1991-1-1:2010 – MALAYSIA NATIONAL ANNEX TO EUROCODE 1: ACTIONS ON STRUCTURES – PART 1-1: GENERAL ACTIONS – DENSITIES, SELF-WEIGHT, IMPOSED LOADS FOR BUILDINGS

This has to be read together with the main document MS EN1991-1-1 because it highlights provisions which allow for local parameters which do not conform exactly to conditions elsewhere. Where required, local design parameters are recommended, otherwise those recommended by BSI will be adopted. For example, fire-fighting and rescue vehicle loads are adjusted accordingly in NA document to reflect local context to differentiate from UK conditions. And AHU floor load is clearly spelt in the NA document – unlike in BS EN version.

6.5 MS EN1992-1-1:2010 – EUROCODE 2: DESIGN OF CONCRETE STRUCTURES – PART 1-1: GENERAL RULES AND RULES FOR BUILDING STRUCTURES

Dr Andrew Minton, of Concrete Centre, UK made the following statement on the application of Eurocode 2 in UK:

"In concrete design it is expected that there will be material cost savings of up to 5% compared with using BS 8110. Furthermore, the Eurocodes are organised to avoid repetition, they are technically advanced and should offer more opportunities for UK designers to work throughout Europe."

The same can be said of Malaysian engineers when they get around to adopting and applying MS EN1992 Eurocode 2 in the local context.

MS EN1992-1-1 gives a general basis for the design of structures in plain, reinforced and pre-stressed concrete made with normal and lightweight aggregates together with specific rules for buildings.

6.6 NA TO MS EN1992-1-1:2010 – EUROCODE 2: MALAYSIA NATIONAL ANNEX TO DESIGN OF CONCRETE STRUCTURES – PART 1-1: GENERAL RULES AND RULES FOR BUILDING STRUCTURES

This has to be read together with the main document MS EN1992-1-1 because it highlights provisions which allow for local parameters which do not conform exactly to conditions elsewhere. Wherever required, local design parameters are recommended, otherwise those recommended by BSI will be adopted. The code drafters intended to produce non-contradictory complementary index (NCCI) for two specific areas of local requirements, which are:

- Thin-sized structural elements in concrete beams and columns in single- and double-storey structures.
- Band beam design for large span concrete floor area.

6.7 MS EN1993-1-1:2010 – EUROCODE 3: DESIGN OF STEEL STRUCTURES – PART 1-1: GENERAL RULES AND RULES FOR BUILDING STRUCTURES

For steel designers: Once they become familiar with the appropriate MS EN1993 documents (a significant task which includes the many parts of the Eurocodes, the national annexes and other

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support information), and familiar with the layout of the clauses within the Standard, the process will be reassuringly similar to design to BS 5950.

This general observation has some exceptions, such as combined axial loads and bending, but with some thought, it is easy to see that the underlying principles of structural mechanics are similar in both standards.

6.8 NA TO MS EN1993-1-1:2010 – EUROCODE 3: MALAYSIA NATIONAL ANNEX TO DESIGN OF STEEL STRUCTURES – PART 1-1: GENERAL RULES AND RULES FOR BUILDING STRUCTURES

This has to be read together with the main document MS EN1993-1-1 because it highlights provisions which allow for local parameters which do not conform exactly to conditions elsewhere. Where required, local design parameters are recommended, otherwise those recommended by BSI will be adopted. For more details of the local parameters used, readers are advised to purchase the relevant National Annex from Standards Malaysia, to get the full information.

6.9 MS EN1997-1:2012 – EUROCODE 7: GEOTECHNICAL DESIGN – PART 1: GENERAL RULES

By way of introduction, Andrew Harris and Dr Andrew Bond of Geomantix, UK have this to say on the Eurocode 7:

"For many geotechnical engineers across Europe, Eurocode 7 represents a major change in design philosophy, away from the traditional allowable (a.k.a. permissible) stress design involving a single, lumped factor of safety. The use of a single factor to account for all uncertainties in the analysis – although convenient – does not provide a proper control of different levels of uncertainty in various parts of the calculation.

"A limit state approach forces designers to think more rigorously about possible modes of failure and those parts of the calculation process where there is most uncertainty. This should lead to more rational levels of reliability for the whole structure. The partial factors in Eurocode 7 have been chosen to give similar designs to those obtained using lumped factors – thereby ensuring that the wealth of previous experience is not lost by the introduction of a radically different design methodology. The Eurocodes present a unified approach to all structural materials and should lead to less confusion and fewer errors when considering soil-structure interaction."

MS EN 1997 applies to the geotechnical aspects of the design of buildings and civil engineering works. It is concerned with the requirements for strength, stability, serviceability and durability of structures. It covers the following topics:

- Basis of geotechnical design
- Geotechnical data
- · Supervision of construction, monitoring and maintenance
- Fill, dewatering, ground improvement and reinforcement
- Spread foundations
- Pile foundations
- Anchorages
- Retaining structures.

6.10 NA TO MS EN1997-1:2012 – EUROCODE 7: MALAYSIA NATIONAL ANNEX TO GEOTECHNICAL DESIGN – PART 1: GENERAL RULES

This has to be read together with the main document MS EN1997-1-1 because it highlights provisions which allow for local parameters which do not conform exactly to conditions

elsewhere. Where required, local design parameters are recommended, otherwise those recommended by BSI will be adopted. For more details of the local parameters used, readers are advised to purchase the relevant National Annex from Standards

FEATURE

7.0 CONCLUSION

Malaysia, to obtain the full information.

The IEM Position Paper published in 2004, specified the need to adopt Eurocode EC2 or, to be more precise, EN1992:2002 as the basis to draft the Malaysian Standards National Annex.

The intention was to institute a transition period which was proposed at 3 years, as agreed by BEM, IEM and ACEM. During this transition period, both Eurocode 2 and BS8110 can co-exist and be used as submission standards. The proposed transition period was from 1st June 2014 to 31st May 2017.

IEM, through its Civil & Structural Engineering Technical Division, has been actively organizing seminars and courses since 2009 on Eurocodes especially on Eurocode EC2 (Concrete Design) and Eurocode EC3 (Steel Design). Such activities will be continued intensively during the transition period, in collaboration with Standards Malaysia.

All the measures to enable a smooth transition to Eurocodes have to be implemented so that the practicing structural engineers can work within the required safety and economical needs of built design structures. This can be done with the co-operation and understanding of all relevant stakeholders in the construction industry.

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Ir. Prof. Dr Jeffrey Chiang is currently Honorary Treasurer of IEM. He has previously served as Honorary Secretary of IEM and is still serving as a member of IEM Technical Division of Civil & Structural Engineering, and as Chairman of IEM Technical Committee on Earthquake. He is also the Dean of Faculty of Engineering & Built Environment at SEGi University, Kota Damansara Campus.

Reliability Analysis for Concrete Structures



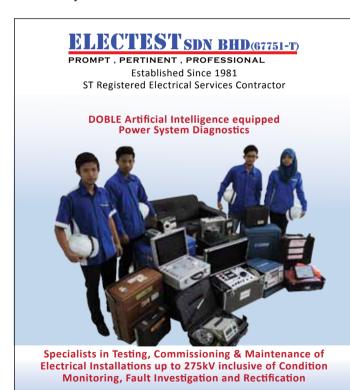
by Ir. Tu Yong Eng

1.0 INTRODUCTION

In structural engineering design, the concept of reliability was relatively new, although it was contained implicitly in most of the British Standards used before in the construction industry. However, it is now explicitly stated in the Structural Eurocodes. In other words, we believe the British Standard was reliable within the scope of its application in the industry, even though the level of reliability was not clearly stated in the codes. In EN 1990 [BSI 2004', the concept of reliability can be found in section 2.2 and Annexes B and C.

Reliability in the context of structural design, is the ability of the designer to create a structure that fulfills the intended design purposes and satisfies the design requirements within the working life span of that structure. The objectives of this paper are as follows:

- i. Explain the concept of reliability of the structure
- ii. Establish the components of reliability and estimate the reliability level for material



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ST Registered Electrical Services Contractor up to 500kV 3-G, 3-1, 3-2 Jalan C U20/C, Section U20, TSB Commercial Centre Sungai Buloh, 40160 Shah Alam, Selangor, Malaysia. Tel: +603 6151 9495 Fax: +603 6151 3930 Email: info@myelectest.com iii. Compute the level of reliability based on the Eurocode EN 1992-1-1 provision for design concept.

Reliability is a statistical concept and is closely related to design return period. The concept of return period has always been misunderstood and confused with the design working life span of the structure. There are various terminologies which are also being used synonymous with return period such as Average Recurrence Interval (ARI) and failure probability within certain period.

In order to estimate the reliability level of the designed structure, it is common that Normal Distributions are assumed. The survival probability is defined by the chances the structure will survive within the stipulated time frame. The indicative measurement called a reliability index, β is related to the failure probability P, such that:

Pf = Φ(-β) (Eqn 1)

where Φ is the standard cumulative normal distribution. In a typical example, through findings by previous researchers [Chiang 2012], the reliability of the designed structure is a function of the following (which can be reflected as reliability indices):

- i. Properties of material used,
- ii. Expected loading to be exerted, nature and magnitude;
- iii. Workmanship or skills in implementation, and
- iv. Structural responses due to loading exerted.

2.0 RELIABILITY IN EN 1990

Section 2.2 of EN1990 explains the basic concept of reliability and practical method on how the reliability within assumption of the codes can be achieved. Reliability is customarily specified for a period of one year and for the life span of the structure (normally 50 years) corresponding to the required limit states of design used. In general, different limit states and components exhibit different level of reliability.

Table B2 of Annex B, EN 1990 gives the specific level of reliability for ultimate limit state of design as follows:

Table 1: Reliability indices	for reliability ca	ategories of d	esign structures

Reliability	Minimum value for β		
classes of design structures	1 year reference period	50 years reference period	
RC 3	5.2	4.3	
RC 2	4.7	3.8	
RC 1	4.2	3.3	

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Based on the figure above, we can compute the failure probability using spreadsheet, as in Microsoft Excel. To compute the failure probability based on Equation 1, the formula NORM.S.DIST(β .1) can be used. Conversely, to find the reliability index β from the failure probability, the function NORM.S.INV(p) can be used.

 Table 2: Maximum failure probabilities for reliability categories of design structures

Reliability	Maximum failure probability P _f			
classes of design structures	1 year reference period	50 years reference period		
RC 3	9.96443E-08	8.53991E-06		
RC 2	1.30081E-06	7.2348E-05		
RC 1	1.33457E-05	4.83424E-04		

3.0 RELATIONSHIP BETWEEN THE RETURN PERIOD AND RELIABILITY INDEX

Many designers and researchers have the following common misconceptions about return period:

- i. Return period should be the same as the life span of the structures.
- To obtain data for high return period, we should have as many data as return period and hence, very high return period is irrelevant.

In fact, the return period and life span of the structures are not inter-related and to obtain high return period information, we do not need as many data as the number of years as in the return period. Just to reiterate, the return period is a statistical concept. Hence, sufficient data is required if we are working with return period. Return period T is related to the occurrence probability within a year as follows:

So, for a 2,500-year return period wind speed, $_{w2500}$ m/s is the wind speed that has only a 0.04% chance of exceeding $_{w2500}$ within one year. Similarly, it has 2% chance to have wind exceeding wind speed with 50 years return period, $_{w50}$ within one year.

For the phenomenon of return period, it requires that the condition remains unchanged throughout the study period and the parameter will be considered as a random variable. With these assumptions, the distribution is a binomial distribution. Therefore, we can compute the chances of the value being exceeded within any period as follows:

 $P_n = 1 - (1_{p1})^n$ (Eqn 3) In Equation 3, P_n refer to the probability of exceedance at least once within n year and p1 is the probability of exceedance at least once within 1 year.

Similarly, we can rewrite Equation 3 for reliability index β for phenomenon that remains uniform throughout the study period (such as wind and earthquake) as follows:

 $\Phi(-\beta_n) = 1 - (1 - \Phi(-\beta_1))^n$ (Eqn 4) In Equation 4, β_n refer to the reliability index for a period of n year and β_1 is the reliability index for 1 year.

In structure loading, permanent action will not be distributed as a binomial distribution, and hence will not satisfy Equation 4.

In the article by Tu (2008), the author has computed the chances of exceedance within various period based on the return period. For example, the chances of exceedance within 50 years for a return period of 2,500 years is 1.98%. It has 10% chances of exceedance within 50 years for event with 475 years of return period. For a structure with a life span of 50 years, if the collapse limit state is taken to be 475 year return period seismic event, then it will have a 10% chance of collapse within the life span of 50 years. This is obviously too high.

4.0 RELIABILITY INDEX COMPUTATION FOR CONCRETE STRUCTURES

In order to achieve the necessary reliability, both EN 1992-1-1 and BS8110 [BSI 1997] have built in the necessary factors into various components. For material, characteristic strength was specified instead of the mean strength. In the practice, some confused between the characteristic strength and minimum cube strength. It should be pointed out that due to the different level of industry practices, the level of reliability of the structures will be different from country to country. Comparing the concrete data from Ray et al. [2014] and Chiang et al. [2012], there are huge different shown in the practices in Hong Kong and Malaysia. Therefore, it is the right time for the local industry to think about and take action on the quality of supplied concrete in Malaysia. The comparisons are as follows:

In both BS 8110 and EN1992-1-1, the reliability indexes for material supply are 1.64. On average, the reliability indexes of concrete supply were 3.407 and 1.351 for Hong Kong and Malaysia respectively.

Oceanda	Hor	ng Kong		Malaysia
Concrete Grade	Mean	Standard Deviation	Mean	Standard Deviation
C25			30	4.5
C30	49	5.3	38	5.6
C35	54	5.2	42	4.6
C40	60	6.9	49	6.7
C45	67	6.3		

 Table 3: Comparison of tested compressive strength of supplied concrete

For loading, two factors were introduced, i.e. characteristic loading and load factor. The characteristic loading was defined as the loading for which only certain prescribed probability being exceeded. However, the level of probability was not specified in the code. In the extreme case, nominal value (or average value) would be specified in some designs. For structural responses, partial factor has been introduced. Based on EN 13791, to cater to the workmanship for concrete, a reduction factor of 0.85 was introduced.



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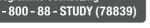
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Example 1:

Consider Ultimate Limit State for a simply supported beam with the following data:

- a) Geometric data:
- i. Length = 6 m
- ii. Width b = 250 mm (Standard deviation of 10 mm)
- ii. Depth h = 600 mm (Standard deviation of 25 mm), cover = 25 mm, link= 10 mm
- b) Material data.
- Concrete C25/30, f_{yk} = 25MPa with standard deviation of 4 MPa;
- ii. Steel reinforcement grade 500B, with $f_y = 500$ MPa with a standard deviation of 35 MPa.
- c) Loading data
- Average permanent action = 31.5 kN/m with a standard deviation of 4.5 kN/m (14.3% of the characteristic loading);
- ii. Average variable action = 10 kN/m with a standard deviation of 3 kN/m (30 % of the characteristic loading)
 First we may ignore the geometric deviation. As

usual, assuming the loading to be normally distributed, characteristic loading will be taken to be the 75 percentile loading. Hence, the loading are as follows:

- a) Characteristic permanent action= 31.5 + 0.67 x 4.5 = 34.5 kN/m
- b) Characteristic variable action = 10 + 0.67 x 3 = 12.0 kN/m
- c) Using partial factors of 1.35 for permanent action and 1.5 for variable action, total design loading is 64.6 kN/m and the design moment generated is 290.7 kNm.
- d) Based on the design, reinforcement required will be 3T25.
- e) Total loading will be distributed according to normal distribution with mean of 41.5 kN/m and standard deviation of 5.41.
- f) Moment due to the loading will be distributed according to normal distribution with mean of 186.75 kNm and standard deviation of 24.34. Hence, the probability of exceeding will be 9.64 x 10⁻⁶.
- g) Mean concrete strength is 25 + 1.64 x 4 = 31.56 MPa;
- h) Mean yield strength of steel bar = 557.4MPa.

In the computation of the capacity of the section, the following formula was adopted (refer to Equation 3.15 and Figure 3.5 of EN1992-1-1, page 34 to 36):

- i. Rectangular stress block was adopted;
- ii. αcc taken to be 0.8 in accordance with the Malaysian National Annex.
- iii. fcd = α cc f_{ck} / γ _c, with the material partial factor γ _c taken to be 1.
- iv. $\lambda = 0.8$ for $f_{ck} \leq 50$ MPa
- v. λ = 0,8 (f_{ck} -50)/400 for 50 < f_{ck} \leq 90 MPa

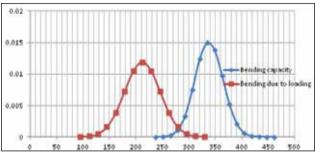
vi. $\eta = 1,0$ for $f_{ck} \leq 50$ MPa

vii. η = 1,0 - (f_{ck} -50)/200 for 50 < f_{ck} \leq 90 MPa

Monte Carlo simulation is carried out to determine the failure possibility of the section. The capacity of the section has a normal distribution with the mean of 398.90 kNm and standard deviation of 23.05. The difference between the capacity and the moment due to loading, is normally distributed with a mean of 212.15 and standard deviation of 33.52. So the reliability of this section under Ultimate

Limit State is 6.329, corresponding to a failure probability of 1.235 x 10 $^{\mbox{-}10}$

Based on EN13791, a reduction factor of 0.85 was introduced for the concrete strength of existing structures. So, taking the reduction factor of 0.85 to the strength of concrete as well as the steel bar, and geometric deviation into consideration, the capacity of the section has a normal distribution with a mean of 338.7 kNm and standard deviation of 26.04. Therefore, the difference between the capacity and moment generated by loading is distributed normally with a mean of 151.95 kNm and a standard deviation of 35.6. The reliability is reduced to 4.263 and the failure probability is 1.0094 x 10⁻⁵. The probability distribution graphs are shown in figure 1 below:



5.0 CONCLUSION

It is important to know the assumed reliability of the specific codes of practices so that the policy maker can determine if the recommendations are too stringent or insufficient. This is especially so for new forms of loading or environmental loading such as adoption of seismic design in Malaysia. As a state-of-the-art codes of practice, Structural Eurocodes have explicitly specified the level of reliability for the structural design and construction. In this paper, an example was presented to illustrate the computation of the reliability of a section design for ultimate limit state.

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He was a member of various technical committees which drafted several Malaysian Standards. He was also an advisor to several local colleges and universities.

Ir. Tu Yong Eng is a professional engineer and amateur mathematician. He has extensive experience in civil and structural, design and construction. He has been continually involved in education and research, participating and organising various courses, seminars and conference.

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Eurocode 3: Design of Steel Structures BS EN 1993-1-

85 EN 1993-1-8: 2005 Design of Joints Journey of the Malaysian Annexe



by Ir. David Ng. Shiu Yuen Chairman for Technical Committee of EC3

Design of Steel structures had come a long way since the introduction of BS5950 which led to a changeover from the Allowable Stress Methods to Limit State Design. This was further advanced with the introduction of new high performance materials, fabrication methods and design which took advantage of the rapid progress and, in particular, the connections. Early connections of steel structures used rivets which were replaced with welded and bolted connections. Developments in automatic fabrications, 3D shop drawings with software connected to numerically controlled machines for laser cutting and punching, have moved away from traditional manual methods.

As a result, higher quality and standardisation are achievable due to automation. Today, steel connections are economical to fabricate and erect, have a high level of safety and results in elegant and practical structures.

To take advantage of the above, Eurocodes standards were developed, taking into account various common design methods available throughout the EU, and consolidated into a single document. This way, a specific steel standard for steel connections was created, unlike the BS5950 which was embedded in the main design document. The EuroNorm (EN) standards on joints have seen a dramatic increase in technical guidance, from a meager 21 pages in BS5950 to 133 pages in the EN 1993-1-8.

The methods used in the British Standards (BS) for connections design were based on a series of capacity checks and there was no guidance or method for determining the connection stiffness and rotational capacity. Recent advancements in research and improved methods have made possible the derivation of stiffness and rotational capacity of bolted and welded connections. EN 1993-1-8 has utilised these advantages and included a series of consistent approach for calculating the strength, stiffness and rotational capacity of a limited and practical range of bolted and welded connections. The resistance of the joints is determined on the basis of the resistances of its basic components and the fasteners with higher shear stiffness should be designed to carry the design load.

The method is called the Component Approach and uses the behaviour of the individual component within a connection (bolts, weld, end plate, columns flange, etc) to simulate the connection's load-deformation characteristic. This allows the designer to predict the behaviour of simple, continuous and semi continuous steel frames. This method is based on research done on flush and extended end-plate and has been extended to include joints with angle cleats, composite and base plates.

In addition to beam to columns connections, EN 1993-1-8 also includes design methods for column bases with end plate connections, new rules for the interactions of moments and axial forces at the connections, new rules for calculating the bearing capacity of slotted holes, welded connections to tubes and improved serviceability limits for pins.

There are some subtle differences in the EN 1993-1-8, in the detailing limitations such as maximum bolt spacing (max. 16t to 14t for all conditions), holes sizes (for calculations of shear capacity – no reduction if less than 1mm clearance for M12 and M14), no edge distance rules when the holes are not in the direction of the forces.

These differences should not have much impact on the connection resistance compared to other standards, including the withdrawn BS5950. Having materials factor EN1993-1-8. said that the in i.e. ym2 = 1.25 for materials, means the joints should have a more conservative value compared to BS5950. In the overall joint capacity calculations the materials factor in the United Kingdom (UK) National Annexe (NA) for BS EN1993-1-1, y0 = 1.00 and ym2 = 1.10 (EN 1993-1-1, ym2 = 1,25). This translates to more economical sections and a heavier connection as opposed to BS5950 where ym = 1.00, regardless of whether it is sections or joint capacity. This is a significant departure in terms of reliability between sections and joints.

In the UK NA, on which the Malaysian National Annexe (NA) will be modelled closely, the connections classifications of pinned and rigid can be based loosely on the SCI publications (NCCI – non-contradictory complementary information) and

this assertion can greatly reduce or eliminate complicated calculations. The influence of NA in the EN document is critical and should not be overlooked as it can make significant changes to the Eurocode core.

The group of committee members in the Malaysia NA drafting committee has a major role in balancing the local available resources and new developments. This is to ensure we have an NA document that is relevant to the industry and, at the same time, pays homage to the original document which was painstakingly prepared by a panel of world experts.

Ir. David Ng. Shiu Yuen is the current Chairman for Technical Committee of EC3.

IEM DIARY OF EVENTS

One-Day Course on Drilling Engineering Overview					
Organised by	: Oil, Gas and Mining Technical Division				
Date	: 20th September 2014				
Time	: 8.30 a.m. – 5.00 p.m.				
Venue	: Wisma IEM				
CPD/PDP:	: 6.5				

 One Day Symposium on Protection Against Lightning

 Organised by
 : Electrical Engineering Technical Division

 Date
 : 27th November 2014

 Time
 : 9.00 a.m. - 5.00 p.m.

mile	. 9.00 a.m. – 5.00 p.m.
Venue	: Grand Dorsett, Subang, Malaysia
CPD/PDP:	: 6.0

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.



After Dark, Making Tracks in The Ground

In the dead of night, when most people are at home sleeping, more than 20 people are 30m below Jalan Bukit Bintang in an underground tunnel. These are the tunnel engineers and ground crew who are working to complete the Mass Rapid Transit (MRT) line.

"We have two 12-hour shifts and the night shift starts from 7pm to 7am,"said MMC-Gamuda yard foreman Mohd Zakiuddin Ridzuan, 31. "Initially, we needed some time to adjust but now, we are already used to it."

Workers have to walk in a narrow 1.1km walkway because it is the only way to reach the tunnel boring machine (TBM) and the tunnel is equipped with electricity to light the path for them.

The work of this overnight tunnelling team certainly looks old-fashioned hard labour because they have to brave dirty water with particles in the air entering their lungs. With the sound of generators running, it feels like an artillery barrage echoing in this nocturnal subterranean world when everyone is shouting over it.

Travelling down through the circular shaft to the pit bottom, precision is key when it comes to cutting out the tunnels using a tunnel boring machine. "We are using a variable density TBM because it is designed to dig through karstic limestone, which is more complex as it contains pockets of water and large void chambers that can cause sinkholes on the surface," said MMC-Gamuda construction manager Yusni Shahadan, 48.

The danger with water while tunnelling is that if the TBM were to hit a water pocket, the pressure would force the water out and may flood the area, Zakiuddin said.

"The high pressure of the water gushing out could cause a lot of damage. It can even affect the TBM and tunnelling will have to be stopped, delaying the MRT construction," he said.

(Sourced from Malay Mail, 9th August 2014)

Petronas is Best Employer

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PETRONAS has won the 5th Asia's Best Employer Brand Awards 2014 in Singapore, cementing the oil and gas multinational's position as one of the most reputable employers in the industry and region.

The award recognised Petronas for its efforts and investment in enhancing its human capital development, as well as human resource practices, policies and strategies towards offering appealing value proposition to increasingly diverse talent segments.

Petronas senior vice-president of group human resource management Raiha Azni Abdul Rahman

said: "Petronas has always believed in 'building own timber' to ensure we have a sustainable pool of talent to support the future growth and expansion of our businesses."

The group is currently working on a number of initiatives to encourage educational foundations and capability development of its talent as well as extending this to the local community as part of its corporate social responsibility in building pipeline of educated citizens.

"We continuously leverage on our own learning institutions such as Petronas Leadership Centre to nurture business-oriented leaders, Institut Teknologi Petroleum Petronas to build technical skills, Universiti Teknologi Petronas (UTP) to produce oil and gas technical graduates and research technologies and Akademi Laut Malaysia (ALAM) as a Centre of Excellence for maritime education and training," she said.

Asia's Best Employer Brand Awards were jointly hosted by Employer Branding Institute, World HRD Congress and Stars of the Industry Group, supported by CHRO Asia as strategic partner and endorsed by Asian Confederation of Businesses.

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(Sourced from NST, 2nd August 2014)

Romania Invites Malaysian Firms to Invest In Infrastructure, Construction Projects

Romania is inviting Malaysian companies to invest in its infrastructure and construction projects like railways and highways.

Its State Secretary for Global Affairs Carmen Liliana Burlacu said: "Malaysia is doing very well in these areas. We would like to see its technology being implemented in Romania as well, specifically in infrastructure and in energy sectors."

The other sectors that had been opened up for foreign investment were agriculture, processed food, tourism, electrical and electronic sectors as well as oil and gas, she added.

Burlacu said foreign investments could either be undertaken through joint ventures or via 100 per cent ownership.

Romania is also keen to share its expertise in energy sector including nuclear, thermo, hydro, wind and solar technology.

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(Sourced from NST, 2nd August 2014)

Malaysia and Japan Team Up To Produce Animation Series

A multimedia super corridor (MSC) status company, Creatvtoon Studios Sdn Bhd

(Creatvtoon) and renowned Japanese content provider P.I.C.S. Co Ltd (P.I.C.S.) have teamed up to produce an animation series.

A memorandum of understanding was signed here by P.I.C.S president/chief executive officer Shinichiro Nakaso and the founder/director of Creatvtoon, Ayie Ibrahim.

Shinichiro said the cooperation is aimed at bringing together the expertise of both companies to produce creative content that will use storylines from Malaysia and Japan.

The cooperation is the result of the initiative by the Multimedia Development Corporation (MDeC), under its Malaysian Creative Industry Policy.

.....

(Sourced from Bernama, 27th August, 2014)

Local Demand For Highend Condominiums Expected to Weaken

Local demand for high-end condominiums is expected to weaken in the next 12 months, primarily in the Kuala Lumpur City Centre (KLCC) area, with more developers anticipated to tap foreign markets.

Jones Lang Wootton Executive Director Malathi Thevendren said the projection is based on the mismatch in the level of pricing by developers with local affordability.

"There are not many people who can actually afford to buy the high-end condominiums. Some developers are even holding back their launches... (as) they know there's no market," she said.

For affordable condominiums, Malathi said demand will see steady growth with young couples and professionals continuing to drive the market.

(Sourced from Bernama, 27th August, 2014)

Icon wins RM297m Ship Supply Pact

Icon Offshore Bhd has won RM297 million contracts to supply a workboat and two anchor handling tug and supply vessels. Its unit Icon Bahtera (B) Sdn Bhd has inked a deal to provide one accommodation work boat to Zell Transportation Sdn Bhd, with Brunei Shell Petroleum as the end customer.

Another unit, Icon Offshore Group Sdn Bhd recently received a letter of intent to extend a contract from an international oil and gas service provider to provide two anchor handling tug and supply vessels.

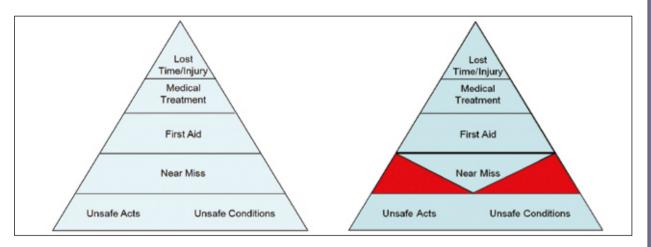
(Sourced from NST, 15th August 2014)





by Ir. Shum Keng Yan

Remember the Safety Triangle (Accident Triangle)? In reality, for normal companies, the triangle will often look like a rocket due to under-reporting of Near Misses in the red zone (refer to February and March 2010 articles).



Now that you are armed with the knowledge of Group A and Group B incidents (May and July 2014 articles), perhaps we need to re-evaluate the focus we pay to Near Miss. Not all Near Misses are the same! Trying to get the numbers to make a triangle should not be the priority.

Many companies use Near Miss as a proactive (leading) indicator due to convenience and also because "others" are using it. By far, Near Miss is a reactive (lagging) indicator. An accident has happened and by chance, has not resulted in injuries. It is only by chance that Near Miss has not turned into a Recordable Case. Think of Near Miss as the technical term for "luck". You can cut and dice and re-arrange the theories but the bottom line remains that the accident has occurred and it is just lucky that there are no injuries.

Now if we are to balance our focus, perhaps we should focus on what is called the Potentially High Impact (PHI or □) Near Misses. They are the Near Misses that can lead to Group A incidents! PHI events need to be identified and investigated as vigorously as Recordable Cases.

This does not mean we can ignore the other cases. It just means that we need to allocate our resources using a more prudent approach.

Here are 5 tips on how to further analyse your data:

- 1. Analyse a minimum period of 5 years (or two long-term plan cycles).
- Look at "high" seasons, corresponding with peak periods of activity.
- 3. Try to see if there is a trend of certain incidents happening at certain time.
- 4. Get an independent person to go through your analysis to take away biases.
- 5. Share your data with the line management for validation.
- 6. Step back and see if there is a bigger pattern emerging from all the incidents.

To share your trends, email us at *pub@iem.org.my* All leading indicators are actually derived from a lagging event. It is how far ahead that we want to go, that defines our measure of proactiveness.

The safest risk is the one that you did not take. Often it is the gap in the risk perception that leads to a gap in risk control.

Ir. Shum Keng Yan is a chemical engineer and a certified accident prevention and safety practitioner. He advises on EHS in the chemical, fast moving consumer goods, heavy metal manufacturing and building services industries across Asia Pacific and beyond. He regularly delivers talks at conferences, forums and universities.

Effects of Thunderstorm Winds in Determining Suitable Wind Speeds for Design of Building Structures in Peninsular Malaysia and Actions



by Ir. Prof. Dr Jeffrey Chiang, Ir. Tu Yong Eng

1.0 INTRODUCTION 1.1 THUNDERSTORM WINDS

1.1.1 Definition of A Thunderstorm

A thunderstorm, also known as an electrical storm, a lightning storm, thundershower or simply a storm, is a form of weather characterised by the presence of lightning and its acoustic effect on the Earth's atmosphere, known as thunder. [Ref. 3].

According to Holmes [Ref. 4], thunderstorms - both isolated storms and those associated with advancing cold fronts – are small disturbances in horizontal extent, compared with extratropical depressions and tropical cyclones. But they are capable of generating severe winds, through tornadoes and downbursts.

Thunderstorms are usually accompanied by strong winds, heavy rain and, sometimes snow, sleet, hail or no precipitation at all. In tropical climate zones such as Malaysia, thunderstorms are associated with a burst of heavy rain and strong winds for a short period of time.

Strong and severe thunderstorms may rotate and this is a phenomenon known as super-cells. While most thunderstorms move with the mean wind flow through the layer of the troposphere that they occupy, vertical wind shear may cause a deviation in their course at a right angle to the wind direction.

1.1.2 Formation of Thunderstorm and Its Effect on Strong Wind Generation

Thunderstorms result from the rapid upward movement of warm, moist air. They can occur inside warm, moist air masses and at fronts. As the warm, moist air moves upward, it cools, condenses and forms cumulonimbus clouds which can reach heights of over 20 km.

When the rising air reaches dew point, water droplets and ice form and begin falling down towards the surface of the Earth. As the droplets fall, they collide with other droplets and become larger. The falling droplets create a downdraft of air that spreads out on the surface of the Earth and causes strong winds associated with thunderstorms.

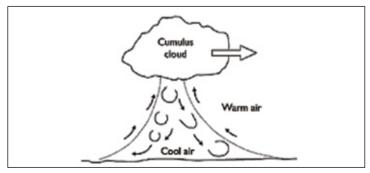


Figure 1: Thunderstorm downdraft [Ref. 4]

Thunderstorms can generally form and develop in any geographic location, perhaps most frequently within areas located at mid-latitude when warm moist air collides with cooler air. Thunderstorms are responsible for the development and formation of many severe weather phenomena. Thunderstorms and the phenomena that occur in relation to them, pose great hazard to populations and landscapes. The resulting damages are mainly caused by downburst winds, large hailstones and flash flooding caused by heavy precipitation. Stronger thunderstorm cells are capable of producing tornadoes and waterspouts.

1.1.3 Classification of Thunderstorms

There are four types of thunderstorms: Single-cell, multicell cluster, multi-cell lines, and super-cells. Super-cell thunderstorms are the strongest and the most associated with severe weather phenomena. Mesoscale convection systems formed by favourable vertical wind shear within the tropics and subtropics, are responsible for the development of hurricanes. Dry thunderstorms, with no precipitation, can cause the outbreak of wildfires from the heat generated from cloud to ground. Several methods are used to study thunderstorms, such as weather radar, weather stations and video photography. The use of Doppler's Effect technology has enabled meteorologists to determine the magnitude and direction of approaching high winds and even to ascertain the vertical wind profile and its impact on high building structures and towers.

1.2 WHAT CONSTITUTES A SEVERE THUNDERSTORM?

A severe thunderstorm is a term designating a thunderstorm that has reached a predetermined level of severity. Often, this level is reached when the storm is strong enough to inflict wind or hail damage. A storm is generally considered severe if winds reach over 90 km/h (25 m/s), hail is 25 mm in diameter or larger or if funnel clouds and/or tornadoes are reported. The super-cell is the strongest of the thunderstorms and is most commonly associated with large hailstones, high winds and tornado formation.

1.3 WHAT ARE DOWNBURST WINDS AND THEIR HAZARDOUS EFFECT?

Downburst winds can cause a lot of damage to landscapes in a thunderstorm. Generally, these are extremely powerful and are often mistaken for wind speeds produced by tornadoes, due to the concentrated amount of force exerted by their straighthorizontal characteristic. Downburst winds may be hazardous to unstable, incomplete or weakly-constructed infrastructures and buildings. Agricultural crops may be destroyed and plants uprooted. Airplanes and other forms of aviation transportation CORROSION MANAGEMENT





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The conditions for generation of severe thunderstorms are [Ref. 4]:

- Water vapour in the atmosphere at low levels, i.e. high humidity
- Instability in the atmosphere, i.e. a negative temperature gradient with height greater than the adiabatic rate of the neutral atmosphere
- A lifting mechanism that promotes the initial rapid convection

 this may be, for example, in the form of a mountain range
 or a cold front.

1.4 WIND PROFILING THUNDERSTORM

The wind profile for thunderstorm [Ref. 4] can be represented as follows:

$$U(r, z) = \frac{\lambda R^2}{2r} \left(1 - e^{-\left(\frac{r}{R}\right)^2} \right) \left(e^{-\frac{z}{c}} - e^{-\frac{z}{k}} \right)$$
 Eqn. (1)

- r = the radial coordinate from the centre of the downburst
- R = the characteristic radius of the downburst shaft
- z = the height above ground level
- c = a characteristic height out of the boundary layer
- k = a characteristic height in the boundary layer
- λ = a scaling factor, with dimensions of [time]⁻¹.

From simple mathematical calculus, the maximum wind velocity U occurs at r = 1.075 R. (Note: In Holmes' postulation, the maximum value occurs at r = 1.121 R). The first two terms in Equation 1 relate to the distance from the centre, while the third term relates to the vertical wind profile. The maximum horizontal wind velocity usually occurs at 50-100 m above the ground level.

The wind profile sketch shown in the following Figure 2, for = 75 m/s, is an arbitrary value for comparison purposes. Further research and site measurement are needed to determine the actual value for λ , c and k.

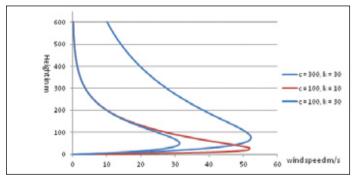


Figure 2: Profile of horizontal wind velocity during a stationary downburst

2.0 CURRENT WIND DESIGN PRACTICE IN MALAYSIA

Local structural engineers are expected to refer to MS1553:2002 [Ref.1] to ascertain the design wind speed which, in turn, will lead to a design wind pressure to be applied as wind loading or actions against building structures. This standard is an adoption of the previous Australian and New Zealand Standard AS1170.2 – 2002 on wind loads.

The wind profile adopted by MS 1553: 2002 is based on power law and is reproduced as follows (in Figure 3):

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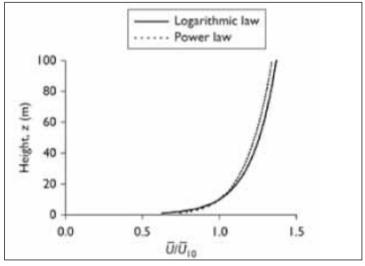


Figure 3: Comparison of the logarithmic and power laws for mean velocity profile Source: John Holmes [Ref. 4]

From Figure 3, we can see that the two relationships are extremely close, and that the power law is quite adequate for engineering purposes.

As discussed above, thunderstorm wind is actually very frequent in Malaysia. By comparing the two sets of the wind profile, that suggested by MS 1553 tends to underestimate the wind effect by up to about 100 m. This will cause problems to low-rise buildings and structures such as billboards or advertisement monopoles. In fact, this was captured by the Thailand wind code, where an additional factor was adopted for advertisement billboard design.

Statistical analysis for MS 1553 was carried out based on the speed of wind, whereas in BS 6399, EN1991-1-4 and Hong Kong wind code, the statistical analysis was carried out for the square of the speed, v^2 .

Furthermore, in MS 1553, only up to a 100-year return period wind speed was quoted. No further information was revealed in the code for the higher return periods. It was also noticed that the statistical analysis was carried out station by station. This will affect the computation of the statistical factor c_{prop} , where every station may have to adopt different figures.

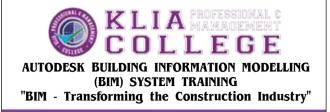
At the moment, the Malaysian Standard is up for a review and changes are expected, not only in putting in the necessary amendments in line with various changes made to the latest revision of AS1170.2, but also the basic wind speeds as presented for the various wind speed stations. All three authors of this paper are members of the reviewing Technical Committee looking into amending MS1553:2002.

3.0 QUESTION OF RELIABILITY AND ACCEPTABILITY OF WIND SPEEDS RECORDED

A working group was formed to look into the accuracy and reliability of wind speed records and how the data can be collated and analysed, so as to produce a distribution of extreme wind speeds which can be relied on for structural design purposes.

Characteristics of the wind – random – in terms of co-relation (linear regression), the trend line should be near horizontal (i.e. the phenomena is consistent throughout the period of study) and R^2 should be practically zero (i.e. to show that the process is actually random). These two criteria are used to check the reliability of the recorded wind speed data.

For some stations, the gradient of the trend lines were negative. By checking on development in the vicinity of a station, it was found



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Further info, please call: Lela/Fakri at 603-8783 1002/1006 Email: lela@kliacs.com.my / fakri@kliacs.com.my Course will be conducted at KLIA Professional & Management College that when the area changed from open space to developed area, the wind speed recorded was eventually reduced over the study period and some stations showed changes in measuring equipment as well as change in location. Hence, suitable adjustment had to be carried out to ensure that the data collected satisfied the consistency and random conditions.

4.0 BASIS OF STUDY INTO EXTREME PROBABILITY ANALYSIS OF RECORDED WIND SPEED DATA

A primary purpose in plotting a set of ordered observations on probability paper is to simplify the inspection of their distribution. Normally, Gumbel Type 1 distribution will be used for the return period analysis. There are a few ways to estimate the probability of exceedance. First of all, we need to arrange the annual extreme in ascending sequence. In general, the probability of exceedance can be estimated by:

$$PE = \frac{n-k}{N+1-2k} Eqn. (2)$$

Equation (2) is valid for k-values not more than 0.5, where n is the ranking of the data and N is the total number of data. Typical values selected for a = 0, 0.44 and 0.5. Gumbel distribution is defined as follows:

$$F(x) = \exp(-\exp(-\alpha(x-u)))$$
Eqn. (3)

Mean
$$\mu = u + \frac{0.577216}{\alpha}$$
 Eqn. (4)

Standard deviation
$$\sigma = \frac{\pi}{\alpha\sqrt{6}}$$
 Eqn. (5)

The parameters α and u will be estimated based on two different methods, i.e. linear regression and maximum likelihood method.

Return period analysis was carried out for data collected from a total of 34 wind stations for both v and calculated v². Data from a total of 37 wind stations were collected as shown in Table 1. If the available data is more than 30, then it is deemed to be acceptable. If the number of data lies between 25 and 29, it is classified as slightly insufficient. Should it fall between 15 and 24, it is classified as insufficient. If the number of data is less than 15, then it will be discarded. The analysis will be carried out on valid data from all relevant wind stations except those classified as discarded.

When the consistency analysis was carried out, the details of station classification were used. The harmonisation of collected data was applied to all stations to ascertain the probability factor. A common consensus was established since the calculated wind pressure is a function of squared velocity (v²). In turn, the return period analysis should also be carried out using the same velocity function to v² instead of v. The basic wind speed based on v² is shown in Table 1. Likewise, the K-factor in the probability factor is also shown in Table 1.

The probability factor, $c_{_{\text{prop}}}$ which is defined as stated in EN1991-1-4 [Ref. 5]:

$$c_{\text{prop}} = \left(\frac{1-K\ln(-\ln(1-p))}{1-K\ln(-\ln(0.98))}\right)^n \qquad \text{Eqn. (6)}$$

Both BS 6399 and EN1991-1-4 recommended that the value of K to be 0.2 and n to be 0.5. The value n = 0.5 corresponds with the analysis performed on v^2 . The values of K-factors for various stations were shown in Table 1. The average value for K was 0.305. It will be the committee's responsibility to determine this Nationally Determined Parameter (NDP) for MS EN 1991-1-4. Figure 4 below shows the comparison of the value of probability factor, cprop corresponding to K = 0.305 and K = 0.2.

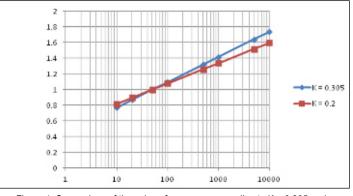


Figure 4: Comparison of the value of cprop corresponding to K = 0.305 and K = 0.2

5.0 OUTCOME OF DATA ANALYSIS FOR VARIOUS WIND STATIONS

5.1 INTERPRETATION OF RESULTS OBTAINED

Thunderstorm wind had not been considered in a logical approach in the previous Malaysian Standard MS1553:2002. The analysis of the collated reliable wind speed data from 34 out of 37 wind stations in Malaysia showed that Kuching in Sarawak has a top wind speed of 35.52 m/s compared to Ipoh which has a recommended wind speed of 35.18 m/s. The previous MS 1553 did not include Sabah and Sarawak in wind speeds of towns/locations there.

The lowest wind speed recommended was 22.28 m/s for Lubuk Merbau, Terengganu, with average wind speed of 29.13 m/s for the country as a whole. In terms of reliability, the K value ranged from a high of 0.63 to a low of 0.18, while the average K was 0.305. The Technical Committee may or may not take the average as the NDP for MS EN1991-1-4.

These results were set to a 3-sec gust wind speed measurement. When the Technical Committee recommends a switchover from

No .	Station Name, State	No. of years	Validity of wind data records	Basic wind speed (m/s)	K-factor
1	Alor Star, Kedah	45	Acceptable	30.17	0.414
2	Batu Embun, Pahang	29	Slightly insufficient	27.97	0.278
3	Batu Pahat, Johor	19	Insufficient data	26.41	0.252
4	Bayan Lepas, Penang	45	Acceptable	26.87	0.217
5	Bintulu, Sarawak	45	Acceptable	25.47	0.216
6	Butterworth, Penang	26	Slightly insufficient	24.93	0.177
7	Cameron Highlands, Pahang	28	Slightly insufficient	28.86	0.249
8	Chuping, Perlis	32	Acceptable	26.23	0.218
9	Gong Kedak, Kelantan	3	Discarded	20.25	0.210
10	Ipoh, Perak	45	Acceptable	35.18	0.399
11	Kapit, Sarawak	4	Discarded		
12	K.Terengganu, Terengganu	26	Slightly insufficient	29.99	0.419
13	Kluang, Johor	37	Acceptable	30.63	0.498
14	Kota Bharu, Kelantan	45	Acceptable	32.58	0.380
15	Kota Kinabalu, Sabah	54	Acceptable	30.60	0.301
16	Kuala Krai, Kelantan	26	Slightly insufficient	29.36	0.226
17	Kuantan, Pahang	45	Acceptable	30.82	0.300
18	Kuching, Sarawak	45	Acceptable	35.52	0.627
19	Kudat, Sabah	30	Acceptable	32.24	0.374
20	Labuan, W. Persekutuan	33	Acceptable	27.45	0.253

Table 1: Location of the wind station and basic wind speed and K-factor

21	Langkawi, Kedah	24	Insufficient data	27.37	0.286
22	Limbang, Sarawak	5	Discarded		
23	Lubuk Merbau, Terengganu	18	Insufficient data	22.28	0.271
24	Melaka, Melaka	45	Acceptable	25.64	0.266
25	Mersing, Johor	45	Acceptable	34.00	0.363
26	Miri, Sarawak	45	Acceptable	29.00	0.272
27	Muadzam Shah, Pahang	28	Slightly insufficient	25.67	0.325
28	Petaling Jaya, Selangor	40	Acceptable	30.08	0.285
29	Sandakan, Sabah	45	Acceptable	26.00	0.229
30	Senai, Johor	37	Acceptable	30.86	0.283
31	Sepang, Selangor	13	Discarded		
32	Sibu, Sarawak	45	Acceptable	30.08	0.241
33	Setiawan, Perak	45	Acceptable	28.39	0.273
34	Sri Aman, Sarawak	28	Slightly insufficient	30.80	0.286
35	Subang, Selangor	45	Acceptable	31.57	0.399
36	Tawau, Sabah	32	Acceptable	28.88	0.263
37	Temerloh, Pahang	33	Acceptable	35.52	0.235
	Maximum value			35.52	0.627
	Minimum value			22.28	0.177
	Average value			29.13	0.305
	Standard			2.99	0.093
	deviation				

FEATURE

MS1553:2002 to MS EN1991-1-4, then a 10-min averaging wind speed will be used in place of the conventional 3-sec gust wind speed, which is more suited to thunderstorm winds. A scaling factor will have to be applied then. This needs further work to be carried out by the IEM Technical Committee on Wind Code.

6.0 CONCLUSION

Based on the discussion above, the following factors shall be taken into consideration for the future Malaysian wind code:

- i. Type of wind the effect of tropical thunderstorm wind shall be considered, especially for medium and low rise structures.
- ii. Basic wind speed computation to adopt the square of the wind speed instead of wind speed
- iii. K value –this factor will affect the reliability of the wind code and requires the Technical Committee to recommend the appropriate Nationally Determined Parameters (NDP) for the MS EN1991-1-4. ■

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Ir. Prof. Dr Jeffrey Chiang is currently Honorary Treasurer of IEM. He has previously served as Honorary Secretary of IEM and is still serving as a member of IEM Technical Division of Civil & Structural Engineering, and as Chairman of IEM Technical Committee on Earthquake. He is also the Dean of Faculty of Engineering & Built Environment at SEGi University, Kota Damansara Campus.

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Preview of National Annex to EC8: Seismic Loadings for Peninsular Malaysia, Sabah and Sarawak



by Ir. Prof. M. C. Hee

INTRODUCTION

In April 2013, a technical paper entitled, Recommended Earthquake Loading Model For Peninsular Malaysia was published in IEM Jurutera. This was followed by a publication in IStructE Conference 2013 (held in Beijing and Shanghai) under the title Earthquake Loading Model In The Proposed National Annex To Eurocode8 For Peninsular Malaysia, both of which were presented by Professor Nelson Lam (refer to Figure 1).

In both papers, the Preliminary Hybrid Seismic Response Spectrum Model for Peninsular Malaysia was presented internationally for the first time.



Figure 1: Front cover of IstructE Conference 2013 paper

From the April 2013 IEM workshop on earthquake and the two technical papers published in 2013, the following are the key points to note:

- 1. The use of 2,500-year return period and displacement based approach for seismic analysis.
- 2. One unified standardised seismic response spectrum model developed for the whole peninsula.
- 3. Two-tier site factor approach to account for local conditions.

In the IEM workshop on "2-day Workshop on Recommended Earthquake Loading Model in the Proposed National Annex to Euro Code 8 for Sabah, Sarawak & Updated Model for Peninsular Malaysia", held on 16th-17th July, 2014, the updated preliminary design spectrum for Peninsular Malaysia, and a newly established preliminary design spectrum for Sabah and Sarawak were introduced.

The report gave a brief introduction into the three response spectra mentioned above, for a return period of 2,500 years of seismic wave transmission at bedrock level.

PENINSULAR MALAYSIA

For the distant earthquake model, the Uniform Hazard Spectrum (UHS) developed by Pappin et al (2011) was modulated to incorporate the latest updates of Ground Motion Prediction Models. For the local earthquake, a deterministic design scenario of M6 at R=20km, based on global hazard model was recommended, reconciling with the world average of intraplate earthquakes for a return period of 2,500 years.

The updated model was revised from the proposed 2013 Design Spectrum Model for Peninsular Malaysia to cater for better understanding of local intraplate earthquakes and modulated corner periods. Combining both distant and local hazards, the updated model was presented in both displacement-based and force-based formats, and expressed algebraically. in a manner similar to EC8 peak ground acceleration format.

Figure 2a below is the displacement spectra, and figure 2b is the acceleration spectra. From the figures, up to period of 2s, the spectra is controlled by the local earthquake, after 2s the distant earthquake controls.

SARAWAK

With reference to Kuching, Sarawak, earthquakes of magnitude 3.5Mb to 5.3Mb have occurred within 500 km from Kuching. There are some major faults in the State, including the Kelwait Fault and Bukit Masing Fault (Figure 3).

A single unified response spectrum for Sarawak is recommended, using the same principle as that for the updated model for Peninsular Malaysia, such that the local and distant models are regulated using the displacement format (Figure 4a) and presented both in displacement and EC8 peak ground acceleration format (Figure 4b). As seen in the figure, the design spectrum for Sarawak is controlled by the local earthquakes.

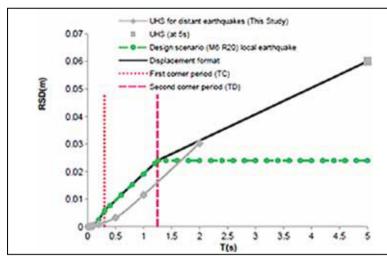


Figure 2a: Displacement format of Design Spectrum model for Peninsular Malaysia

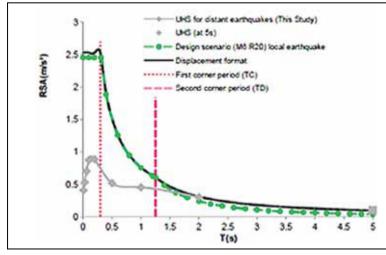


Figure 2b: EC8 peak ground acceleration format Design Spectrum model for Peninsular Malaysia

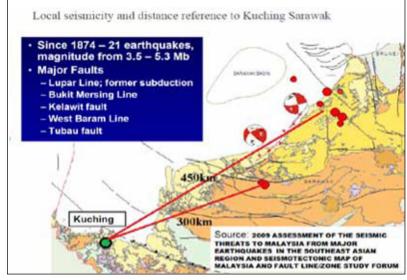
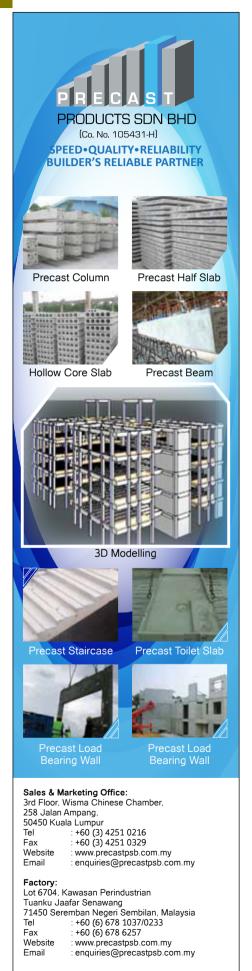


Figure 3: Kuching Reference Distance to fault zones

SABAH

Since Sabah is very near highly volatile, earthquake-prone countries such as Indonesia and Philippines, the chances of an earthquake happening there cannot be taken lightly or overruled. In the past, earthquakes of magnitude greater than 5 have been recorded at a distance of approximately





100 km from Sabah (Figure 5). Hence a unified design model for the whole of Sabah was recommended (Figure 6a/b), similar to that for Peninsular Malaysia and Sarawak.

PRELIMINARY COSTING

Preliminary structural cost implication due to the proposed Hybrid Design Spectrum was carried out on structures of rigid-jointed frame for 1-storey and 5-storey buildings, shear wall-frame for 10-storey and cored-wall for 30-storey buildings.

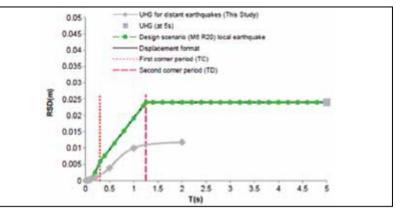


Figure 4a: Displacement format of Design Spectrum model for Sarawak

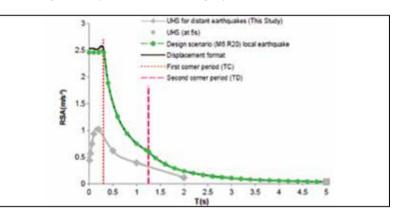


Figure 4b: EC8 peak ground acceleration format Design Spectrum model for Sarawak

Refer to the IEM "2-day Workshop on Recommended Earthquake Loading Model in the Proposed National Annex to Euro Code 8 for Sabah, Sarawak & Updated Model for Peninsular Malaysia",16th-17th July, 2014 lecture notes for further details in the costing.

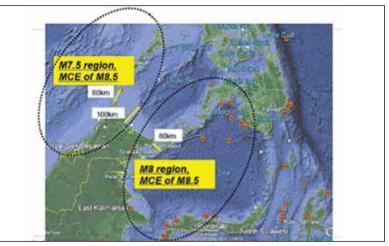


Figure 5: Sabah Reference Distance to fault zones



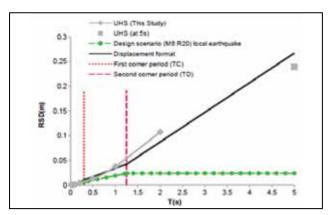


Figure 6a: Displacement format of Design Spectrum model for Sabah

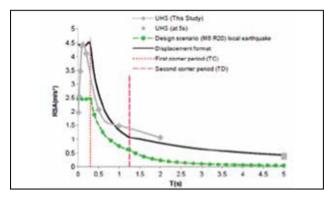


Figure 6b: EC8 peak ground acceleration format Design Spectrum model for Sabah

RECOMMENDATION

The recommended earthquake design spectrums presented at the workshop are as summarised below in Table 1.

Table 1. Parameters for Hybrid Design Spectrum for P. Malaysia, Sabah, Sarawak

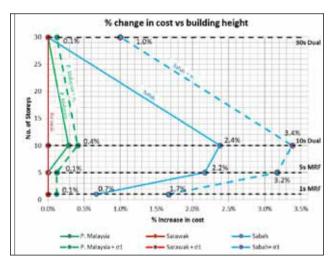


Figure 7: Preliminary structural cost increase (excluding foundation)

Parameter	RSA _{max}	PGA	SD(T _D)	Slope m	T _c	T _D
Unit	g	g	mm	mm/s	s	s
P. Malaysia	0.25	0.1	24	9.6	0.3	1.25
Sarawak	0.25	0.1	24	0	0.3	1.25
Sabah	0.45	0.18	42	60	0.3	1.25

Refer to the IEM "2-day Workshop on Recommended Earthquake Loading Model in the Proposed National Annex to Euro Code 8 for Sabah, Sarawak & Updated Model for Peninsular Malaysia", 16th-17th July, 2014 lecture notes for further details.

CONCLUSION

It can be concluded that:

Hybrid Design Response Spectrum for Peninsular Malaysia is a combination of short and long distant earthquake with a peak ground acceleration of 0.1g for a return period of 2500 years at bedrock level.

- Hybrid Design Response Spectrum for Sarawak is dominated by local earthquake with a peak ground acceleration of 0.1g for a return period of 2,500 years at bedrock level.
- Hybrid Design Response Spectrum for Sabah is dominated by long distant earthquake with a peak ground acceleration of 0.18g for a return period of 2,500 years at bedrock level.
- Preliminary structural cost increase (excluding foundation) is only 3.4% for a 10-storey building. ■

Ir. Prof. M.C. Hee is a member of IEM TechnicalCommittee on Earthquake, Working Group 1 Chairman, Study on Peak Ground Acceleration and Response Spectrum for Malaysia.

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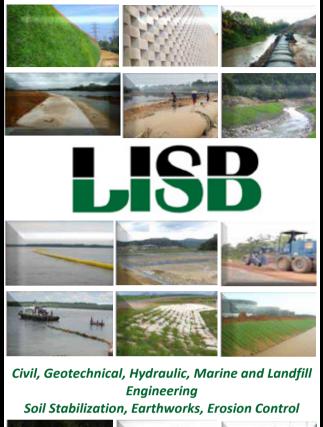
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Half-day Seminar on "Impact of **Structural Eurocodes on Steel and Composite Steel-Concrete** Structures"



by Ir. Dr Na Soon China

CIVIL & STRUCTURAL ENGINEERING TECHNICAL DIVISION

THE Civil and Structural Engineering Technical Division (CSETD) of IEM and Hiap Teck Venture Berhad (HTVB) Group of Companies jointly organised a half-day seminar titled Impact Of Structural Eurocodes On Steel and Composite-steel Concrete Structures. The seminar, sponsored by HTVB, was held at Sunway Resort Hotel on 22nd November 2013 and attended by 95 participants.

The speaker was Associate Professor Dr Chiew Sing-Ping, an expert on steel structures from the School Of Civil And Environmental Engineering, Nanyang Technological University, Singapore. He was also the Past President of the Singapore Structural Steel Society, former Council and Executive Member of the Institution of Engineers, Singapore and Vice-Chairman of the Joint Structural Division of the Institution of Structural Engineers.

Dr Chiew started by talking about Structural Eurocodes which will be fully implemented and mandatory in Singapore by 1st April 2015. The two-year transition period started on 1st April 2013 and ends 31st March 2015 when both British Standard (BS) and Structural Eurocodes are acceptable in the city-state. He believes the implementation of Structural Eurocodes in Malaysia is underway and advises design engineers to start preparing for this as soon as possible.

In the first section of the seminar, Dr Chiew focused on the impact of Eurocode 3 (EC3) on steel structures design. A few design aspects coupled with working examples were highlighted, such as the residual stress due to the imperfections of materials, shear buckling for S460 steel, web bearing and web buckling. In the second section, he focused on the impact of Eurocode 4 (EC4), Composite Steel and Concrete Structures. He compared EC4 and BS in materials strength specification. In composite structures, the resistance of headed shear stud connectors is generally lower in EC4 as compared to BS5950.

Dr Chiew also discussed the guidance for lateral-torsional buckling check for continuous composite beams and prototype testing and development of composite slab system using new profiled steel sheeting as per EC4.

In the Q & A session that followed, he took questions raised by the participants and the seminar ended at 1.00 p.m. CSETD representative Ir. Lo Seng Ling presented a memento and certificate of appreciation to Dr Chiew. All participants were given a complimentary copy of Design Of Composite Steel And Concrete Structures With Worked Examples To Eurocode 4, which was authored by Dr Chiew and his colleague. The event continued with a factory visit to HTVB in Klang by 22 participants.

Ir. Dr Ng Soon Ching is an Assistant Professor at Universiti Tunku Abdul Rahman (UTAR). He serves as a committee member in IEM Civil and Structural Engineering Technical Division.

Two-Day Course on "Simplified-Unified Practical Design to MS EC0, MS EC1 and MS EC2 from A Consulting Engineer's Perspective"

CIVIL & STRUCTURAL ENGINEERING TECHNICAL DIVISION

THE Civil & Structural Engineering Technical Division organised a two-day course on "Simplified-Unified Practical Design To MS EC0, MS EC1 and MS EC2 from a Consulting Engineer's Perspective" on 17th-18th October 2013, at Armada Hotel in Petaling Jaya, Selangor.

The course instructor, Ir. M.C. Hee, is a Consulting Structural Engineer, Principal of MC Hee & Associates, committee member of the Civil and Structural Engineering Technical Division, and Adjunct Professor at University of Malaya.

The course attracted 95 participants, including consultant engineers, engineers from contracting firms, government agencies and local authorities, as well as faculty members from local institutions of higher learning. There were both young and experienced engineers in the audience. Ir. Ong Sang Woh chaired the session on the first day, and Ir. Boone Lim chaired the session on the second day.

DAY ONE

Ir. Hee started with a lecture on the fundamentals of MS EC0: Basis Of Structural Design, by stressing on the configuration, relationship and the scope of various Structural Eurocodes, as well as on the impact of Eurocodes for structural designers. Among the topics he covered were the terminologies used in Eurocodes, partial safety factors for loads and materials, fundamental load combinations for ULS and SLS conditions, examples of Ψ values for different load combinations, vertical and horizontal deflection limits and the division of braced and unbraced frame structure for analysis. Various work examples were presented to enhance the participants' understanding. Ir. Hee stressed that unlike conventional design codes which are descriptive in nature, the Eurocodes are performance based.

Then, he deliberated on MS EC1: Actions on Structures, i.e. the loading code. The topics covered included classifications of actions and action categories with reference to the tables provided in the National Annex of EC1. Ir. Hee also presented examples of calculation for imposed loads reduction on beams and columns, with comparisons between the requirements of National Annex of EC1 and CP3 codes.

In the third lecture, he discussed the SLS requirements of durability and fire design according to MS EC2 as well as the factors causing durability loss in structural elements, code requirement for minimum concrete cover for various exposure classes (to meet durability requirements) and standard fire resistance.

Several practical examples were presented and comparison was made to the cover requirements in BS8110. Ir. Hee then talked about Design of RC Beams by using the "unified approach" in accordance to MS EC2. Unlike the conventional approach of analysis and design of beams, the unified approach utilises the factor of combined reinforcing index, q (as shown in Eq. 1), where the q factor can be used as a unifying parameter in the process of designing reinforced, pre-stressed and partially pre-stressed sections. The other topics discussed during this session included:

FORUM

by Engr. Dr Sudharshan N. Raman, Grad. IEM

- Type of beam failure and beam ductility
- Design of singly and doubly reinforced RC rectangular beams, design of T or L section beams
- Comparison of flexural capacity between BS8110 and EC2

Next he revisited the fundamentals of structural analysis. He strongly encouraged the use of manual check methods such as two-cycle moment distribution to acquire the feel of how structures behave under applied loadings by qualitative analysis in sketching the deflected profile and the bending moment diagrams. Examples were shown to demonstrate the application of two-cycle moment distribution, in combination with the Principle of Virtual Displacement (PVD), to verify computer results and sketching the deflection profiles of beams. He also covered the code coefficients for beam and one-way slab as provided in BS8110 and EC2 design codes, and the division of braced and unbraced frames during the analysis process.

The final lecture dealt with the SLS requirement for deflection and cracking. Among the topics discussed were the deemed-tocomply method, the K factors for structural systems, effective spanto-depth ratios and the practical applications of modification factors for span-to-depth ratios.

DAY TWO

There were four main lectures today as follows:

- 1. Simplified shear design to MS EC2
- 2. Design of RC columns in accordance to MS EC2
- 3. Design of plain and RC walls to MS EC2
- 4. Why we get it wrong sometimes.

Under the topic "shear design", Ir. Hee covered the fundamentals of shear in beams, shear capacity of sections with and without reinforcements (longitudinal and shear), computations for web crushing approach and how to deal with concentrated loads located close to support.

Various worked examples comparing the shear design according to BS8110 and EC2 were presented during the lecture, and the question and answer session proved to be an enriching experience for the participants.

The second lecture was on the design of RC columns in accordance to EC2. Ir. Hee started with a discussion on column concrete cover requirement for fire and durability. He then covered the nuts and bolts related to the analysis and design of RC columns,

 $\begin{array}{l} q=\!0.567[1\!-\!\!\sqrt{1-\frac{2\kappa}{0.567}} \quad]\!=\!0.87q\!-\!0.87q^2\!=\!0.567\gamma k\!\ \dots 1; \gamma\!=\!0.8; \ (0,204\!-\!0.057) \\ \\ For k_{u,max}\!=\!0.45; \ \gamma\!=\!0.8; \ q_{max}\!=\!0.204 \ \text{and} \ K_{max}\!=\!0.167 \\ \\ For k_{u,min}\!=\!0.125; \ \gamma\!=\!0.8; \ q_{min}\!=\!0.057 \ \text{and} \ K_{min}\!=\!0.054 \end{array}$

K= $\frac{M_{Ed}}{hd^2 f}$ q(1 $\frac{0.5q}{0.565}$) for no compression steel;

Kmax=0.167 and Kmin=0.054



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such as column rebar detailing, the computation of effective length and slenderness ratio of columns, the use of ISE Manual in the analysis and design of stocky and slender columns as well as L-shaped column design principles.

He also introduced the definitive concept to distinguish whether a column was considered braced or unbraced, i.e. by using the ACI Commentary statement (as mentioned below) and the stability index of the frame. Many participants acknowledged that this was the first time that they were introduced to a technique to definitively "calculate the brace-ability of columns". Several worked examples were also presented to reinforce the participants' understanding of these concepts.

The speaker then invited Mr. Zuhal, his understudy, to demonstrate the use of a computer programme in the design computation for some examples presented in this session.

$|f \Sigma k bracings > 6\Sigma k$ columns, then the frames are braced, therefore the columns are also braced

The third lecture started with the structural classification of walls and on the concrete cover requirement to meet fire resistance and durability needs. This was followed by a discussion on the various aspects related to the analysis and design of plain and RC walls.



Ir. M.C. Hee speaking at the course

Some examples included braced or unbraced classification of walls, effective length of plain walls, capacity of plain walls, capacity of stocky and slender RC walls, interaction diagram for RC core walls, etc.

Just as in the other sessions, Ir. Hee presented numerous practical examples to strengthen the participants' understanding of the application of code requirements. In addition, demonstrations were carried out for the treatment of selected RC wall design problems with a computer programme.



Ir.Hee (right) receiving a token of appreciation from Ir. Boone Lim (left)

To conclude the two-day session, the last slot was an interactive session between Ir. Hee and the participants on "Why we get it wrong sometimes?" Ir. Hee presented various cases of common errors that could be committed by an engineer, thus compromising the design, and then provided a guide on how these "errors" could be avoided.

The participants also took the opportunity to share some of the design and construction challenges they had encountered in their practice and to get the feedback from Ir. Hee.

In closing, Ir. Boone Lim thanked Ir. Hee for delivering such a comprehensive and valuable course. Then, on behalf of the organisers, he presented Ir. Hee with a token of appreciation.

Engr. Dr Sudharshan Naidu A/L Raman is currently a committee member of the Civil and Structural Engineering Technical Division. (CSETD).

2-Day Workshop on "Recommended Earthquake Loading Model in The Proposed National Annex (N.A.) to EC8 for Sabah, Sarawak and update for Peninsular Malaysia"



by Ir. Mun Kwai Peng and Ir. Dr Ooi Heong Beng (IEM TC for EC8)

CIVIL & STRUCTURAL ENGINEERING TECHNICAL DIVISION

THE Civil and Structural Engineering Technical Division organised a 2-day workshop attended by 80 participants at Armada Hotel, Petaling Jaya, Selangor, on 16th & 17th July 2014.

It was presented by Professor Nelson Lam (Professor and Reader, University of Melbourne), Ir. Professor M.C. Hee (Consulting Structural Engineer, WG1 IEM TC for EC8) and Engr. Daniel Looi T.W. (Ph.D. Candidate, University of Hong Kong, WG1 of IEM TC for EC8).

DAY 1

1.1. WORKSHOP COMMENCEMENT

The Chairman of the Technical Committee (TC) for Earthquake Engineering in IEM, Prof. Ir. Dr Jeffrey Chiang, started with an introductory briefing into the work done by the committee for Earthquake Engineering in IEM. He briefed the participants on the background work, the structure of the committee, various working groups (WG) and the targeted completion of EC8 of the TC on Earthquake Engineering by IEM. The committee is responsible for studying the effects of Eurocode EC8 on reinforced concrete buildings in the event of an earthquake.

Next, Encik Hassal Zamir, representing the Director of the Department of Standards (DSM), officially opened the event and gave a short speech on the work and background of the department. The workshop was sponsored by the DSM.

1.2. BACKGROUND SEISMICITY OF LOCAL INTRAPLATE EARTHQUAKES

The first presentation was delivered by Prof. Nelson Lam on the background seismicity of local intraplate earthquake, using land mass and occurrences of past earthquakes for 50 years, to arrive at the most appropriate predicted maximum of magnitude earthquake of M6 at a distance R of 20km.

He then demonstrated ground motion response predictions that lead to recommended design peak ground accelerations (pga) values for intraplate earthquakes. The maximum pga for 2,500-year Return Period at 0.1g (g is the acceleration due to gravity at 9.81m/s2) would be appropriate for Peninsular Malaysia. The effect of structures due to earthquake should be analysed and checked using all 3 types of Spectrum namely acceleration, velocity and displacement response spectrums.

1.3. RECOMMENDED APPROACHES FOR MODELLING SITE EFFECTS IN LOCAL EARTHQUAKE

After a short tea break and networking session, Prof. Lam delivered the second presentation titled "Recommended

approaches for modelling site effects in local earthquake". He presented the approach to obtain the design spectrum for design of structures and explained the effects of soil amplification and the importance of soil properties to achieve the response spectrum. He showed how various corners in the response spectrum were established and explained how the results were affected by soil properties and depth of soil. He strongly recommended that EC8 does not recommend a realistic value and one using the number, should take precautions on local soil effect. Natural period of the soil layer is the most important parameter in constructing the response spectrum for design. Prof. Lam also explained how to compute the natural period with the use of shear wave velocity and how to use the average shear wave velocities through layers of different soil with different soil properties.

1.4. UPDATED DESIGN SPECTRUM FOR PENINSULAR MALAYSIA

After lunch, Engr. Daniel Looi T.W. presented the Updated Design Spectrum for Peninsular Malaysia on bedrock for a Return Period of 2,500 years. Engr. Looi is currently pursuing his PhD in Hong Kong University on Earthquake Engineering. The main parts of the study on response spectrum due to far field earthquake and local earthquake effect on structures in the peninsula was explained in detail in a technical paper published in the IEM Bulletin April 2013. The updated response spectrum using a Hybrid Modelling approach, which is a combination of local and distant earthquakes, was explained in detail and presented in both displacement based format and EC8 PGA format. The updated version of the response spectrum will be published soon in the IEM Bulletin.

1.5. PRELIMINARY COST IMPLICATION FOR UPDATED DESIGN SPECTRUM FOR PENINSULAR MALAYSIA

Ir. Prof. M.C. Hee presented the findings for cost comparison between normal EC2 design for reinforced concrete and seismic design, using EC8 and the proposed Hybrid response spectrum for Peninsular Malaysia. He used examples for 1-storey, 5-storey, 10-storey and 30-storey reinforced concrete buildings (figure 1a to figure 1d respectively). The cost comparison presented was only for the structural components. The cost of finishes, M&E and foundation were not included. The increase in construction cost due to seismic design requirement according to EC8 and the proposed Hybrid response spectrum for Peninsula Malaysia may be minimal and insignificant.



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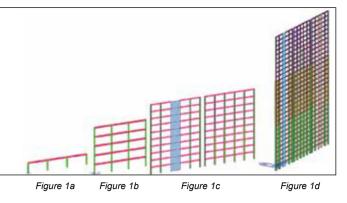


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1.6. DESIGN SPECTRUM FOR PENINSULA SARAWAK

The last agenda of the day was delivered by Engr. Looi who presented the philosophy and concepts used to analyse the seismic effect in Sarawak, which was similar to that for the peninsula.

He presented the recommended Response Spectrum of Sarawak (at bedrock) in both displacement format and EC8 PGA (or spectral acceleration) format for a Return Period of 2,500 years.This was a single unified seismological model across Sarawak (similar to the case in Peninsular Malaysia,with no Distance Factor).From this, he concluded that the response spectrum for Sarawak was controlled by local earthquake.

DAY 2

2.1 COST IMPLICATION FOR DESIGN SPECTRUM FOR SARAWAK

Ir. Prof. Hee recapped what was discussed on the 1st Day:

- a) The set of preliminary structural design of reinforced concrete buildings of height a) 1-storey, b) 5-storey, c) 10-storey, and
 d) 30-storey, based on updated loading (action) model (Design Spectrum) for Peninsular Malaysia.
- b) The structural layouts consist of rigid-jointed frame for 1-storey and 5-storey buildings, shear wall-frame for 10-storey and cored-wall for 30-storey buildings.
- c) To model the non-linear behaviour of buildings under seismic action, in accordance with EC8-1, 4.3.1(7), the stiffness of the load bearing elements were evaluated, taking into account the effect of cracking. The elastic flexural and shear stiffness properties of concrete elements were taken to be one-half of the corresponding stiffness of uncracked elements.
- d) Unit cost of construction adopted: Formwork at RM 40/m2, C30/37 Concrete at RM 300/m3 and Rebar at RM 3.5/kg.

The Design Spectrum (DS) for Sarawak, as presented in Day One, was used to apply seismic actions on the 4 types of RC Buildings.

There was no cost increase of the design inclusive of earthquake action, compared with the design load combination without earthquake action. Load combination with imposed Notional Load (action) controlled the design.

A copy of the proceedings will be kept in the IEM Library. From his presentation, he concluded that for Sarawak, there was no increase in costing.

2.2. AN ALTERNATIVE STATIC METHOD OF ANALYSIS OF BUILDINGS

Prof. Lam presented a Quasi-static (Capacity spectrum) analysis method, assuming linear elastic behaviour of the building.

Lateral Force distribution of arbitrary amplitude is applied. Resulting deflection distribution is calculated. The "effective" mass and stiffness can then be obtained to compute the fundamental natural frequency of the system (eq.2). If the

SUMMARY OF DESIGN BASIS			
Design	MS EN	Eurocode	Remarks
Basis of Structural Design	MS EN 1990	EC0	
Actions	MS EN 1991-1-1	EC1	
Wind Actions	MS EN 1991-1-4	EC1	(Basic Wind Speed=20m/s, Terrain Category 4- City Areas)
Wind Eccentricity	MS EN 1990, 3.5.8(P); MS EN 1991-1-4, 7.1.2	EC0 EC1	(15% perpendicular to wind direction)
Notional Load (action)	MS EN 1991-1-6, Annex A1, A1.3	EC1	Nominal Horizontal Forces
Reinforced Concrete Design	MS EN 1992-1-1 EC2	EC2	
Seismic Design	MS EN 1998-1	EC8	
Seismic Action	MS EN 1998-1 (National Annex)	EC8 NA	Malaysia Annex, Hybrid DS Model_(IEM TC Proposed)
Seismic Eccentricity	MS EN 1998-1, 4.3.2	EC8	(5% perpendicular to seismic direction)

applied force is v_b on a structure with storey mass m_r , producing a deflection at each level δj , the effective stiffness k_{eff} and effective period T_{eff} can be calculated using the following formulas;

$$m_{eff} = \frac{\sum (m_j \delta_j)^2}{\sum m_j \delta_j^2}; \ \delta_{eff} = \frac{\sum m_j \delta_j^2}{\sum m_j \delta_j}; \ k_{eff} = \frac{v_b}{\delta_{eff}}$$
(Equation 1)
$$T_{eff} = 2\pi \sqrt{\frac{m_{eff}}{k_{eff}}}$$
(Equation 2)

The results from this method were compared with those calculated using the EC8 (Annex B) procedure, which used the "equivalent" mass and stiffness approach. The resulting natural period only differs by less than 5%.

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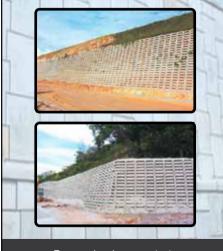
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2.3. PRELIMINARY RECOMMENDED DESIGN SPECTRUM FOR SABAH

Engr. Looi presented the recommended Response Spectrum of Sabah (at bedrock) in both, displacement format, and EC8 PGA (or spectral acceleration) format for Return Period of 2,500 years

This is a single unified seismological model across Sabah (similar to the case in Peninsular Malaysia and Sarawak with no Distance Factor).

This model had been verified against Ground Motion Prediction Models for Long Distance Earthquakes developed by Megawati and Pan, from a catalogue of major long distance earthquakes.

This model had also been verified against Jack Pappin's UHS model, which was developed by ARUP, 2011 for long distance hazards (Return Period of 2,500years).

From the response spectrum presented, he concluded that the response spectrum for Sabah was controlled by distant earthquake.

2.4. PRELIMINARY COST IMPLICATION FOR DESIGN SPECTRUM FOR SABAH

Adopting the same structural layout and cost data used for Peninsular Malaysia, Ir. Prof. Hee applied the proposed seismic DS Model action for Sabah to obtain the seismic design and quantities.

Similarly allowing for a 1% standard deviation of cost results, the structural design, including seismic action, had incurred a maximum cost increase of 3.4%, in comparison with non-seismic design.

2.5. DISPLACEMENT BASED METHOD OF ASSESSMENT

Conventional seismic design is forced-based. However, the real behaviour of structure in a severe earthquake is related to material strains and structural stability, which are functions of displacement or out-of-plumb. Displacement as opposed to Force, is a more representative indicator of structural performance under seismic action. Prof. Lam began his presentation by explaining force calculation based design approach, deformation calculation based design and deformation specification based approach. After that, he explained the importance of displacement based design and why it is preferred. He presented several case studies to illustrate such displacement-demand controlled failure modes.

The risks of overturning can be assessed accurately based on comparing Displacement Demand against Displacement Capacity.

"Size Effects" have been identified as a controlling factor, and this cannot be captured by the conventional force-based procedure.

2.6. SUMMARY OF PRELIMINARY COST IMPLICATION

In the last presentation, Ir. Prof. Hee summarised the structural cost comparison for Reinforced Concrete Building Design of different heights (1-storey, 5-storey, 10-storey and 30-storey) with their prevailing respective type of construction (rigid frame, rigid frame, frame-shear wall, cored-wall) for the peninsula, Sarawak and Sabah. The seismic design was subject to proposed preliminary Hybrid DS Model action corresponding to their respective locations.

In his summary, Sabah DS (Design Spectrum) resulted in largest structural cost increase of 3.4%, compared with non-seismic design. Sarawak and Peninsular Malaysia DS did not result in significant cost increase by including seismic consideration in the design.

Note that the non-seismic structural design (based case) are all based on Eurocodes.

The set of British Standards for structural design (such as BS 8110, BS 5950) has been withdrawn by British Standards Institution since 31st March 2010.

2.7. OPEN FORUM (THIS SESSION WAS INSERTED PER REQUEST OF UTM GROUP OF RESEARCHERS)

An open forum was arranged for the end of the workshopto record discussions or dissent to the IEM TC Proposed Hybrid DS Model presented.

None of the participants raised objections to the recommended Earthquake Loading Model for Sabah, Sarawak and Updated Model for Peninsular Malaysia as presented in the workshop.

In the earlier workshop session, participants from UTM claimed that much work have been done by the UTM group of research workers. The fact is that hardly any peer-reviewed research result has been published or made available for comparison with the findings of IEM EC8 Malaysia Annex Technical Committee.

In the absence of dissent, the IEM Technical Committee will regard the Proposed Hybrid DS Model as having being endorsed by the workshop participants. This is in order to progress further towards finalising a Malaysia National Annex.

Prof. Lam stressed that it was very important that research results be peer-reviewed and published, so that they could be available for verification and review by other researchers. Any deficiencies or defects could then be corrected and the results upgraded.

Ir. Mun Kwai Peng is currently a member of the Technical Committee (TC) on Earthquake (Eurocode 8).

Ir. Dr Ooi Heong Beng is presently an Assistant Professor in the Civil Engineering Department of Universiti Tunku Abdul Rahman. He graduated from University of Malaya in 1972 with BEng (Hons, 1st Class). After 3 years of compulsory service in JKR, he went to study MSc from University of London, a DIC (Concrete Structure and Technology) and PhD from Imperial College, London. He is a registered Professional Engineer with the Board of Engineers, Malaysia and Member of the Institution of Engineers, Malaysia and Institution of Structural Engineers, UK.

He began his career as a Bridge and Road Design Engineer with PWD (Malaysia) and later entered the private sector as Technical and R&D Manager in the piling industry and then progressed to Lead Civil / Structural Engineer in Ranhill Bersekutu before progressing to OGP Technical Services Sdn Bhd (a subsidiary of PETRONAS) and Ranhill Worley Sdn. Bhd.

He is presently a Member of the Technical Committee (TC) on Earthquake (Eurocode 8), Working Group 4 of TC Earthquake, Working Group 1 of TC Concrete (EC2) and participated in various conferences, seminars and workshops on Eurocodes.

IEM DIARY OF EVENTS

Technical Talk: The Niched Role of Traction Power System in the Electrical Power Engineering Profession for Malaysia's Electric-based Railway Public Transportation Infrastructure Projects

30th September 2014

Organised by	: Electrical Engineering Technical Division
Time	: 5.30 p.m. – 7.30 p.m.
Venue	: Wisma IEM
CPD/PDP:	: 2

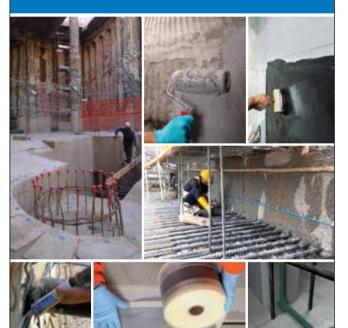
Talk on Fixing of Fees by Professional Bodies under the Competition Act 2010 - Issues and Impact

19th September 2014

Organised by	: Standing Committee by Professional Practise
Time	: 5.30 p.m. – 7.30 p.m.
Venue	: Wisma IEM
CPD/PDP:	: 2

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SENARAI CALON-CALON YANG LAYAK MENDUDUKI TEMUDUGA PROFESIONAL TAHUN 2014

Berikut adalah senarai calon yang layak untuk menduduki Temuduga Profesional bagi tahun 2014.

Mengikut Undang-Undang Kecil IEM, Seksyen 3.9, nama-nama seperti tersenarai berikut diterbitkan sebagai calon-calon yang layak untuk menjadi Ahli Institusi, dengan syarat bahawa mereka lulus Temuduga Profesional tahun 2014.

Sekiranya terdapat Ahli Korporat yang mempunyai bantahan terhadap mana-mana calon yang didapati tidak sesuai untuk menduduki Temuduga Profesional, surat bantahan boleh dikemukakan kepada Setiausaha Kehormat, IEM. Surat bantahan hendaklah dikemukakan sebulan dari tarikh penerbitan dikeluarkan.

Ir. Gunasagaran Kristnan

Setiausaha Kehormat, IEM,

PERM	IOHONAN BARU	KEJURUT
Nama	Kelayakan	26496
KEJURUTERAAN AEROANGKAS	A	
IZA IRYANI BINTI PAUZAN	BE HONS (UITM) (CIVIL, 2009)	KEJURUT
		54510
KEJURUTERAAN KOMPUTER & M	COMUNIKASI	41988
NORHANA BINTI ARSAD	BE HONS (UPM) (COMPUTER SYSTEM & COMMUNICATION, 2000) MSc (UPM) (COMMUNICATION & NETWORK, 2003)	66462
KEJURUTERAAN ELEKTRIKAL		KEJURUT
ABDUL YASIM BIN ALI	BE HONS (UITM) (ELECTRICAL, 2001)	41157
ABU BAKAR BIN HASAN	BE HONS (UTM) (ELECTRICAL, 2011),	
	MSc (CRANFIELD INSTITUTE OF TECH) (1984) PhD (UNITEN) (2013)	
ARMAN BIN CHE OTHMAN	BE HONS (UTM) (ELECTRICAL, 2001)	KEJURUT
LEE PAK FU	BE (SOUTH AUSTRALIA) (ELECTRICAL & MECHATRONIC, 2001)	46771
MARIANNE SHAMINI A/P CHRISTOPHER GEORGE	BE HONS(UNITEN) (ELECTRICAL & ELECTRONICS, 2005)	69489
MGT. NASARUDDIN BIN MGT. MAZELAN	BE HONS (UPM) (ELECTRICAL & ELECTRONICS, 2005)	
MOHD FADLI BIN OTHAMAN	BE HONS (UITM) (ELECTRICAL, 2006)	IEM
MOHD SYAHIR KYAIRI BIN AHMAD FUAD	BE HONS (UKM) (ELECTRICAL & ELECTRONIC, 2005)	One-l
MUHAMAD HAFIS BIN SANURI	BE HONS (UTM) (ELECTRICAL, 2007)	
MUHAMMAD NASEER BIN ALIAS	ME HONS (WALES) (ELECTRICAL & ELECTRONIC, 2008)	20th S Organi
PARAMESWARY A/P ALAGAN	BE HONS (UTM) (ELECTRICAL, 2002)	•
SHARIZAL BIN SHAMURI	BE HONS (WALES) (ELECTRICAL & ELECTRONIC, 1997) MSc (STRATHCLYDE) (ELECTRICAL POWER WITH BUSINESS, 2004)	Time Venue CPD/Pl
KEJURUTERAAN ELEKTRONIK		
MOHD KHAIRUDDIN BIN MD ARSHAD	BE HONS (UTM) (MECHATRONICS, 1999)	Half [
	MSc (UKM) (ELECTRICAL,	Subm
	ELECTRONIC & SYSTEMS, 2005) PhD (LOUVAIN) (2013)	Jubii
		20th 9
KEJURUTERAAN PEMBUATAN		Organi
KHAIRUDIN MOHAMED	BE HONS (UITM) (MECHANICAL, 1995)	U
KEJURUTERAAN MEKANIKAL		Time
KAZREN JUSRI BIN ABDUL KARIM	BE HONS (UTM) (MECHANICAL, 2003)	
MAHADHIR B. MUSTAFFA	BE HONS (UTM) (MECHANICAL, 2000)	Venue
ROSLENDA BINTI HASAN	BE HONS (UITM) (MECHANICAL, 1999) MSc (UPM) (EMERGENCY RESPONSES & PLANNING, 2000)	CPD/P
ZULKEFLY BIN MOHD YUSOF	2009) BE HONS (UITM) (MECHANICAL, 1999) MSc (UPM) (EMERGENCY RESPONSES & PLANNING, 2009)	Half I and E
		25th 9
KEJURUTERAAN ARKITEK NAVA		Organi
YONG WU YUNG, DANIEL	BE HONS (LONDON) (NAVAL ARCHITECTURE & OCEAN, 1997)	Time
	002.11, 1007)	
KEJURUTERAAN TELEKOMUNIK	ASI	Venue
YAN NAZIHA BINTI MD DALIB	BE HONS (MULTIMEDIA) (ELECTRICAL, 2002)	CPD/P

YAN NAZIHA BINTI MD DALIB

BE HONS (MULTIMEDIA) (ELECTRICAL, 2002) MSc (COLLEGE LONDON) (TELECOMMUNICATIONS, 2009)

	PERP	'INDAHAN AHLI
No. Ahli	Nama	Kelayakan
KEJURUT	ERAAN KIMIA	
38065	RIDHUAN AFFANDI BIN ABU BAKAR	BE HONS (UPM) (CHEMICAL, 2007)
44580	ZAINAL BIN AHMAD	BE HONS (SURREY) (CHEMICAL, 1998) MSc (NEWCASTLE UPON TYNE) (2001) PhD (NEWCASTLE UPON TYNE) (2005)

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KEJURUTERAAN AWAM

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572	CHAN HOOI SAN	BE HONS (UTM) (CIVIL, 2002)
460	CHEE WENG YAN	BE HONS (UTM) (CIVIL, 2003)
756	HASEEF IDZLAN BIN ZAHIDI	BE HONS (UNISEL) (CIVIL, 2009) MSc (UiTM) (CIVIL, 2013)
135	LIM THIAM POH	BE HONS (UTM) (CIVIL, 2000)
122	MOHD FADLI BIN OSMAN	BE HONS (UPM) (CIVIL, 2003)
126	ONG SHENG HOW	BE HONS (RMIT) (CIVIL, 2000)
093	PETRUS PHUI VUI KIUN	BE HONS (CURTIN) (CIVIL & CONSTRUCTION, 2010)
122	TEOH KEAN TEONG	BE HONS (UMS) (CIVIL, 2005)
219	WONG HOCK TEONG	BE HONS (MALAYA) (CIVIL, 2007)
JURUTI	ERAAN ELEKTRIKAL	
704	ABU THALHAH B. MOHD LAZIM	BE HONS (MALAYA) (ELECTRICAL, 2007)

ME (UNITEN) (2013)

BE HONS (MULTIMEDIA) (ELECTRICAL, 2010) BE HONS (UNITEN) (ELECTRICAL POWER, 2009)

BE HONS (USM) (ELECTRICAL, 2009) MSc (USM) (ELECTRICAL & ELECTRONIC, 2012)

BE HONS (CURTIN) (ELECTRICAL, 06)

BE HONS (UTM) (ELECTRICAL, 2010)

BE HONS (QUEENSLAND) (CIVIL, 2007)

BE HONS (UITM) (MECHANICAL, 2006)

BE HONS (UTM) (MECHANICAL, 2007) ME (UTM) (MECHANICAL, 2011)

BE HONS (NORTH LONDON) (POLYMER, 1996) MBA (UKM) (2000), MSc (UITM) (2005) PhD (UKM) (MATERIALS SCIENCE, 2014)

BE (HONS) (MMU) (ELECTRONIC, 2003)

MSc (MMU) (MASTER OF SCIENCE, 2007)

ME HONS (IMPERIAL COLLEGE LONDON) (ELECTRICAL & ELECTRONIC, 2009)

BE HONS (UTM) (MECHANICAL-AERONAUTICS, 2003)

BE HONS (UNITEN) (CIVIL, 2005)

49537	ENG TERNG GOANG
45347	MAHESVARAN S/O SIBEPEREGASAM
49975	PNG TIANG CHUN, FRANKIE
49586	TAN KHENG KWANG

KEJURUTERAAN ELEKTRONIK 47548 FAIZ BIN MOHD TURAN

KEJURUTERAAN GEOTEKNIKAL

70629	SHAH IZZNI TALIF BIN MOHD ADNAN	

KEJURUTERAAN LEBUHRAYA26496KANTHIMA NUIN

KEJURUTERAAN MEKANIKAL

54510	FAIT BIN HANAPI
41988	MOHD ZIRWANDY BIN ZAINUDDIN
66462	NOORAZIZI BIN MOHD SAMSUDDIN

KEJURUTERAAN POLIMER

41157 MOHD BIJARIMI BIN MAT PIAH

KEJURUTERAAN TELEKOMUNIKASI

46771 CHUA MING YAM

69489 SITI NURBAYA MOHAMED

EM DIARY OF EVENTS

One-Day Course on Drilling Engineering Overview

20th September 2014

Organised by	: Oil, Gas and Mining Technical Division
Time	: 8.30 a.m 5.00 p.m.
Venue	: Wisma IEM
CPD/PDP:	: 6.5

Half Day Workshop on Principal Submitting Person (PSP) / Submitting Person (SP) to Adapt with OSC 3.0

20th September 2014

Organised by	: Consulting Engineering Special Interest Group and	
	Supported by Kementerian Perumahan Dan Kerajaan	
	Tempatan (KPKT)	
Time	: 8.30 a.m. – 1.00 p.m.	
Venue	: Wisma IEM	
CPD/PDP:	:0	

Half Day Seminar on Latest Fire Safety Engineering Design and Best Industry Practice (Postponed from 22 May 2014)

25th September 2014

Organised by	: Young Engineers Section
Time	: 9.00 a.m. – 1.00 p.m.
Venue	: Wisma IEM
CPD/PDP:	: 3.5

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.

KEAHLIAN

Note: This is a continuation of the list which was first published on page 46 of the August 2014 issue.

KEJURUTERAAN ELEKTRONIK

KEJUR	UTERAAN ELEKTRO	NIK
69946	MACOUVER NG	1ST YEAR (UNIMAS) (MECHANICAL)
69947	MAJORIE TELUN AVIT	1ST YEAR (UNIMAS) (MECHANICAL)
69948	MAS ADILAH BT. MAHAMOOD AHAMAD	1ST YEAR (UNIMAS) (MECHANICAL)
69949	MAS NOR ZAMIRLAH BT. ZAMIRHAN	1ST YEAR (UNIMAS) (MECHANICAL)
69875	MASITAH BT. OSMAN	1ST YEAR (UTM) (MECHANICAL)
69876	MEGAVARNAN A/L MAHIVANAN	1ST YEAR (UTM) (MECHANICAL)
69657	MIUHAMMAD BURHANUDDIN HELMI B. AZMI	1ST YEAR (UKM) (MECHANICAL)
69658	MOHAMAD AFIQ B. ZAINUDDIN	1ST YEAR (UKM) (MECHANICAL)
69659	MOHAMAD AMIRUL B. MOHAMAD ZAHIR	1ST YEAR (UKM) (MECHANICAL)
69660	Mohamad Faiz B. Pathurrahman	1ST YEAR (UKM) (MECHANICAL)
69950	MOHAMAD FARID B. A GHANI	1ST YEAR (UNIMAS) (MECHANICAL)
69877	Mohamad Khairul B. Mohd Noor	2ND YEAR (UTM) (MECHANICAL)
69951	MOHAMAD NAZRI B. HANG JEBAT	2ND YEAR (UNIMAS) (MECHANICAL)
69952	MOHAMAD RIZQI IMTIYAZ B. GINDAR	1ST YEAR (UNIMAS) (MECHANICAL)
69953	MOHAMAD RUSLI B. FARALI	1ST YEAR (UNIMAS) (MECHANICAL)
69661	MOHAMAD ZULZAFRI B. MOHD KUSAINI	1ST YEAR (UKM)
69954	MOHAMMAD AMIRUL FAIZ B. KALANA	(MECHANICAL) 1ST YEAR (UNIMAS) (MECHANICAL)
69955	MOHAMMAD	1ST YEAR (UNIMAS)
	ISKANDAR ZULKARNAIN B. ROSLAN	(MECHANICAL)
69956	MOHAMMAD SYAFIQ B. MANAFFERY	1ST YEAR (UNIMAS) (MECHANICAL)
69662	MOHAMMAD SYAHIRAN B. MANSOR	1ST YEAR (UKM) (MECHANICAL)
69957	MOHAMMAD ZULHAFIZ B. JENUREN	1ST YEAR (UNIMAS) (MECHANICAL)
69958	MOHD ASYRAF B. ZAKARIA	1ST YEAR (UNIMAS) (MECHANICAL)
69959	MOHD AZRI B. AZIZI	1ST YEAR (UNIMAS) (MECHANICAL)
69960	MOHD FARIQ AFA B. MATNOOR	1ST YEAR (UNIMAS) (MECHANICAL)
69961	MOHD NIZAM B. IBRAHIM	1ST YEAR (UNIMAS) (MECHANICAL)
69962	MOHD NURAMIN B. MASLI	1ST YEAR (UNIMAS) (MECHANICAL)
69963	MOHD SHAUQI B. ISA	1ST YEAR (UNIMAS) (MECHANICAL)
69964	MOHD SYZWAN ASQLANI B. AHIDI	1ST YEAR (UNIMAS) (MECHANICAL)
69965	MUHAMAD AMIRUL SHAFIQ B. ZAFRI	1ST YEAR (UNIMAS) (MECHANICAL)
69663	MUHAMAD AQIL B. AZRI	1ST YEAR (UKM) (MECHANICAL)
69966	MUHAMMAD ADLI DIAH B. AHMAD ZABIDI	1ST YEAR (UNIMAS) (MECHANICAL)
69967	MUHAMMAD AMMAR ASYRAF B. ZAINUDDIN	1ST YEAR (UNIMAS) (MECHANICAL)
69878	MUHAMMAD FAIZ HILMI B. MOKHTAR DIN	2ND YEAR (UTM) (MECHANICAL)
69664	MUHAMMAD FIRDAUS B. KARIM @ KASIM	1ST YEAR (UKM) (MECHANICAL)
69968	MUHAMMAD FIRDAUS B. SHAHAR	1ST YEAR (UNIMAS) (MECHANICAL)
69969	MUHAMMAD FITRI B. MD RADZI	1ST YEAR (UNIMAS) (MECHANICAL)
69970	MUHAMMAD HAFIZ B. MOHD HAJAD	1ST YEAR (UNIMAS) (MECHANICAL)
69665	MUHAMMAD HAFIZ B. MOHD NOOR	1ST YEAR (UKM) (MECHANICAL)
69971	MUHAMMAD HARMAINEY B. JASNIH	4TH YEAR (UNIMAS) (MECHANICAL)
69666	MUHAMMAD HAZMAN B. MD ISA	1ST YEAR (UKM)
69667	B. MD ISA MUHAMMAD IBRAHIM B. SAMSUDDIN	(MECHANICAL) 1ST YEAR (UKM) (MECHANICAL)
69972	MUHAMMAD MAULA	1ST YEAR (UNIMAS)
69973	NORDIN MUHAMMAD NABIL B.	(MECHANICAL) 1ST YEAR (UNIMAS)
69974	HAS-BULLAH MUHAMMAD NAZIRUL	(MECHANICAL) 1ST YEAR (UNIMAS)
69975	B. HAMDAN MUHAMMAD NAZRIN	(MECHANICAL) 1ST YEAR (UNIMAS)
69976	B. SUFIYAN MUHAMMAD TAUFIQIL	(MECHANICAL) 1ST YEAR (UNIMAS)
69668	hafiz B. Musa Muhammad zaki B.	(MECHANICAL) 1ST YEAR (UKM)
	SHAHBANI	(MECHANICAL)

69977	MUNIERAH BT. SAEE	4TH Y
69677	MUSLIM B. ABDUL	(MECH 4TH Y
69978	HAMID NADZRIN AHMED B.	(MECH 1ST YI
69879	FASIHUDDIN NARESH A/L SIVALINGAM	(MECH 1ST YI
69979	NAZRUL IRFAN B. ABDUL RAHMAN	(MECH 1ST YI
69980	NAZURAH BT. ANTUNI	(MECH 1ST YI
69880	NG YOUNG CHING	(MECH 1ST YI (MECH
69981	NOAMIE ANAK BALMI	1ST YI (MECH
69669	NOOR ATIQAH BT. BADALUDDIN	1ST YI (MECH
69982	NOOR JUWITA BT. JUNAIDI	1ST YI (MECH
69881	NOORHIZATHULAKMA BT. KHAMIS	1ST YI (MECH
69983	NOORUL AQIDAH BT. SHAMSULUDA	1ST YI (MECH
69984	NOR UMIRAH BT. SUDIRMAN	1ST YI (MECH
69985	NUR ADILAH BT. AHMAD	1ST YI (MECH
69986	NUR FARAH BT. ZAIN	1ST YI (MECH
69670	NUR FARAHHUDA BT. ZAINAL ABIDIN	4TH Y (MECH
69671	NUR HAFIZAH BT. HABIDEEN	1ST YI (MECH
69672	NUR HASDYANNA BT. HASNURASHID	1ST YI (MECH
69987	NUR MEHRANIE BT. SHAMSUL BAHRI	1ST YI (MECH
69882	NUR NABILAH BT. MOHAMMAD MUSTAFA	1ST YI (MECH
69988	NUR NADIA BT. HAMRI	1ST YI (MECH
69989	NUR SHARZAINA HANUM BT. SEKIUN	1ST YI (MECH
69883	NUR'AN AMIERA BT. OMAR	1ST YI (MECH
69990	NURUL ADZATUL NADIA BT. ADZAHAR	1ST YI (MECH
69884	NURUL FAZIERA BT. FAIZUL RAHMAN	1ST YI (MECH
69885	NURUL NADIA BT. MOHD IBRAHIM	1ST YI (MECH
69886	NURULHUDA BT. TAJUDDIN	1ST YI (MECH
69887	OOI TZE XIONG	2ND Y (MECH
69888	PANG JACKSON	1ST YI (MECH
69991	PATRICA DO WAN	1ST YI (MECH
69992	PATRICIA IVY ANAK EDWIN AMIN	4TH Y (MECH
69889	PERAVEEN A/L N KANNADHAS	1ST YI (MECH
69993	PHILLIP MAPANG ANAK ANGKING	1ST YI (MECH
69673	RAJA SHARMIN IZZATY BT. RAJA LOB	1ST YI (MECH
69994	SHARUDDIN RAYMOND LUBUN	4TH Y
69995	ANAK TINSANG RIDHWAN B. BUSRAH	(MECH 1ST YI
69890	SHALVEEN DAVE	(MECH 1ST YI
69891	SINGH SHAZLIEN AINI BT.	(MECH 1ST YI
69996	KAULANI SHAZRUL NIZWAN B. SUHARDI	(MECH 1ST YI
69674	SITI AISYAH BT. ISMAIL	(MECH 1ST YI (MECH
69997	SITI FARIS AFIFAH BT. ABDUL GHANI	1ST YI (MECH
69998	SITI FATIMAH BT. JASNI	1ST YI
69999	JASNI STEVEN JOELANAK TOTAL	(MECH 1ST YI (MECH
69892	SYAFIQAH BT. AZMI	(MECH 1ST YI (MECH
69893	SYAHIRA ATIKAH BT. RAMLE	(MECH 1ST YI (MECH
70000	SYLVIA WENDY ANAK BONEY	1ST YI (MECH
69894	TAN MENG TECK	1ST YI (MECH
69895	TANG BING CHUONG, JACKY	1ST YI (MECH
70001	TANG KWONG YONG	1ST YI (MECH
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70002	TERENCE EMPURAI ANAK ANDAP	1ST YEAR (UNIMAS) (MECHANICAL)
70003	THADDELIS SENABONG ANAK RENJAN	1ST YEAR (UNIMAS) (MECHANICAL)
70004	TIE TONG HENG, THOMAS	1ST YEAR (UNIMAS) (MECHANICAL)
70005	TIM BRANDON ANAK SAWING	1ST YEAR (UNIMAS) (MECHANICAL)
70006	TING SING FUI	1ST YEAR (UNIMAS) (MECHANICAL)
69896	TIONG JUANG LOONG	1ST YEAR (UTM) (MECHANICAL)
69897	VAIRASIVAM A/L KARISNA KUMAR	1ST YEAR (UTM) (MECHANICAL)
70007	VIGNESHWARAN A/L RAMRAO	1ST YEAR (UNIMAS) (MECHANICAL)
69649	WILLIAM GRANETIN ANAK THOMAS	3RD YEAR (UTEM) (MECHANICAL)
69675	WINSTON GERANG	2ND YEAR (UKM) (MECHANICAL)
70008	WONG HA LEI, AMBROSE	1ST YEAR (UNIMAS) (MECHANICAL)
69898	WONG TING PHANG	1ST YEAR (UTM) (MECHANICAL)
70009	YASMIN SYAZWANI BT. MOHAMAD	1ST YEAR (UNIMAS) (MECHANICAL)
69899	YONG YUNG YIK	1ST YEAR (UTM) (MECHANICAL)
69900	ZAFRUDDIN B. ABDUL AZIZ	1ST YEAR (UTM) (MECHANICAL)
70010	ZARITH NAJWA BT. ZAIDI	1ST YEAR (UNIMAS) (MECHANICAL)

KEJURUTERAAN MEKATRONIK

69688	AHMAD THUFFAIL THASTHAKEER	1ST YEAR (MONASH) (MECHATRONICS)
69681	AIKA AN NAJWA BT. MOHD MARZUDIN	1ST YEAR (UTEM) (MECHATRONICS)
69682	KAM FOONG YUAN	1ST YEAR (UTEM) (MECHATRONICS)
69678	KARTHIK SURIANDRAN A/L MATHAVAN	1ST YEAR (UTAR) (MECHATRONICS)
69683	LOR SHENG QIN	1ST YEAR (UTEM) (MECHATRONICS)
69680	MOUATOVA ADEL	1ST YEAR (APU) (MECHATRONICS)
69684	NURUL AISHAH BT. ZAMSURI	1ST YEAR (UTEM) (MECHATRONICS)
69685	SITI AISYAH BT. ZAINAL	1ST YEAR (UTEM) (MECHATRONICS)
69686	SITI NORBAIZURA BT. ABDUL AZIZ	1ST YEAR (UTEM) (MECHATRONICS)
69689	SULTAN EISSA EISSA SHWOK	1ST YEAR (MONASH) (MECHATRONICS)
69687	TAN WEI CHIANG	1ST YEAR (UTEM) (MECHATRONICS)
69679	TAN WEI CHOON	1ST YEAR (UTAR) (MECHATRONICS)

KEJURUTERAAN PEMBUATAN

RESOLUTERAART EMBOATAN			
69621	AFIFAH ZAKIYYAH BT. JURI	1ST YEAR (UMP) (MANUFACTURING)	
69622	AMANINA SHARIDZAN BT. AZIZAN	1ST YEAR (UKM) (MANUFACTURING)	
69623	FARAH HAFIZAH BT. HAMKA	1ST YEAR (UKM) (MANUFACTURING)	
69624	FARLENA IDAYU BT. MD SALLEH	1ST YEAR (UKM) (MANUFACTURING)	
69625	ILYAS SYAFIQ B. OTHMAN	1ST YEAR (UKM) (MANUFACTURING)	
69615	KOH CHEE HAO	1ST YEAR (UTAR) (MANUFACTURING)	
69616	LEE HAOZHE	1ST YEAR (UTAR) (MANUFACTURING)	
69617	LEE YI HEAN	2ND YEAR (UTAR) (MANUFACTURING)	
69618	MAH CHIA YI	1ST YEAR (UTAR) (MANUFACTURING)	
69626	MOHAMMAD AFIQ B. ABDUL RASHID	1ST YEAR (UKM) (MANUFACTURING)	
69619	MOHD FUAD B. UMAR	4TH YEAR (UTEM) (MANUFACTURING)	
69627	MOHD SYAMSUL AZWANI B. MOHD ABDULLAH	1ST YEAR (UKM) (MANUFACTURING)	
69628	MUHAMAD MUSTAQIM B. ANDUL LATIFF	1ST YEAR (UKM) (MANUFACTURING)	
69629	MUHAMMAD ALIF B. JAMIAN	1ST YEAR (UKM) (MANUFACTURING)	
69630	MUHAMMAD AMIRUL ASHRAFF B. IHAS	1ST YEAR (UKM) (MANUFACTURING)	
69620	MUHAMMAD AZLAN B. MD AMIN	4TH YEAR (UTEM) (MANUFACTURING)	
69631	Muhammad Fakhrul Razie B. Mamat	1ST YEAR (UKM) (MANUFACTURING)	

KEAHLIAN

69632	MUHAMMAD NOOR FAHMY MAULA FAZARNOR	1ST YEAR (UKM) (MANUFACTURING)
69633	MUHAMMAD SYAZANI FARHAN B. ZAINI	1ST YEAR (UKM) (MANUFACTURING)
69634	NOOR FALHIAH BT. SAARI	1ST YEAR (UKM) (MANUFACTURING)
69635	NORNISAADILA BT. MUSA	1ST YEAR (UKM) (MANUFACTURING)
69636	NUR ANIS FASHA BT. ROSLAN	1ST YEAR (UKM) (MANUFACTURING)
69637	NUR FATIHAH BT. ABDUL GHAFAR	1ST YEAR (UKM) (MANUFACTURING)
69638	NUR FRAHANA BT. MUSTAFFA	1ST YEAR (UKM) (MANUFACTURING)
69639	NUR NABIHAH NAJAI BT. ROZIANUAR	1ST YEAR (UKM) (MANUFACTURING)
69640	NUR SHUHADA BT. AMIN	1ST YEAR (UKM) (MANUFACTURING)
69641	NURUL ADAWIYAH BT. M. JASSIN	1ST YEAR (UKM) (MANUFACTURING)
69642	NURUL IZAAN BT. ABDULLAH HALID	1ST YEAR (UKM) (MANUFACTURING)
69643	NURUL NADIA BT. ISMAIL	1ST YEAR (UKM) (MANUFACTURING)
69644	SITI SYAHARA BT. MAD YUSOH	1ST YEAR (UKM) (MANUFACTURING)
69645	TUAN ZULFADLI SYAMIM B. TUAN MOHD ADNAN	1ST YEAR (UKM) (MANUFACTURING)
69646	UMAR AFIQ B. RAMLI	1ST YEAR (UKM) (MANUFACTURING)

R (UKM) ACTURING)

IEM DIARY OF EVENTS

7th International Conference on Cooling & Heating Technologies 2014 (ICCHT 2014)

4th – 6th November 2014

Innovation And Sustainability In Heating & Cooling Technologies Venue : Dorsett Grand Subang Hotel, Subang Jaya, Selangor

TYPE	REGISTRATION FEE	
	Normal (after 30 Sept 2014)	Early Bird (30 Sept 2014 or before)
Full Delegate	US\$500 (RM1,600.00)	US\$450 (RM1,440.00)
Student	US\$350 (RM1,120.00)	US\$300 (RM960.00)
Accompany guest	US\$200 (RM640.00)	

For more information, please contact ICCHT 2014 Secretariat Tel No : +(603) 7958 6851 Fax No: +(603) 7958 2851 E-mail : iemtrainingcentre@iemtc.com

Website: http://www.myiem.org.my/ICCHT2014

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.

IEM DIARY OF EVENTS

Talk on "Utilization Of EFB **Biomass In Power Generation**"

30th September 2014

Organised by	: Agricultural & Food
	Engineering Technical
	Division
Time	: 5.30 p.m 7.30 p.m.
Venue	: Wisma IEM
CPD/PDP	: 2

Talk On Understanding Risk, **Opportunities & Strategies In Fixed Income Investment**

08th October 2014

Organised by	: Seniors Special Interest
	Group
Time	: 5.30 p.m 7.30 p.m.
Venue	: Wisma IEM
CPD/PDP	:0

IEM DIARY OF EVENTS

Talk On Palm Oil Refinery **Project-Sharing Of Experience**

11th	Octobe	r 2014
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Organised by	: Chemical Engineering
	Technical Division
Time	: 9.00 a.m 10.59 a.m.
Venue	: Wisma IEM
CPD/PDP	: 2

28th Annual General Meeting Chemical Engineering Technical Division

11th October 2014

: Chemical Engineering	
Technical Division	
11.00 a.m 01.00 p.m.	
Wisma IEM	
2	

IEM DIARY OF EVENTS

Seminar on Deep Excavation

13th October 2014

Organised by	: Geotechnical Engineering Technical Division and IEM Sarawak Branch
Time	: 8.00 a.m 5.00 p.m.
Venue	: Wisma IEM
CPD/PDP	: 6.5

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.

CONTRIBUTIONS TO WISMA IEM BUILDING FUND



RM 2,391,491.70 from IEM Members and Committees RM 741,502.00 from Private Organisations TOTAL RM 3,132,993.70

(ANOTHER RM 4,615,829.31 IS NEEDED)

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2.7. OPEN FORUM (THIS SESSION WAS INSERTED PER REQUEST OF UTM GROUP OF RESEARCHERS)

An open forum was arranged for the end of the workshopto record discussions or dissent to the IEM TC Proposed Hybrid DS Model presented.

None of the participants raised objections to the recommended Earthquake Loading Model for Sabah, Sarawak and Updated Model for Peninsular Malaysia as presented in the workshop.

In the earlier workshop session, participants from UTM claimed that much work have been done by the UTM group of research workers. The fact is that hardly any peer-reviewed research result has been published or made available for comparison with the findings of IEM EC8 Malaysia Annex Technical Committee.

In the absence of dissent, the IEM Technical Committee will regard the Proposed Hybrid DS Model as having being endorsed by the workshop participants. This is in order to progress further towards finalising a Malaysia National Annex.

Prof. Lam stressed that it was very important that research results be peer-reviewed and published, so that they could be available for verification and review by other researchers. Any deficiencies or defects could then be corrected and the results upgraded.

Ir. MunKwai Peng is currently a member of the Technical Committee (TC) on Earthquake (Eurocode 8).

Ir. Dr Ooi Heong Beng is presently an Assistant Professor in the Civil Engineering Department of Universiti Tunku Abdul Rahman. He graduated from University of Malaya in 1972 with BEng (Hons, 1st Class). After 3 years of compulsory service in JKR, he went to study MSc from University of London, a DIC (Concrete Structure and Technology) and PhD from Imperial College, London. He is a registered Professional Engineer with the Board of Engineers, Malaysia and Member of the Institution of Engineers, Malaysia and Institution of Structural Engineers, UK.

He began his career as a Bridge and Road Design Engineer with PWD (Malaysia) and later entered the private sector as Technical and R&D Manager in the piling industry and then progressed to Lead Civil / Structural Engineer in Ranhill Bersekutu before progressing to OGP Technical Services Sdn Bhd (a subsidiary of PETRONAS) and Ranhill Worley Sdn. Bhd.

He is presently a Member of the Technical Committee (TC) on Earthquake (Eurocode 8), Working Group 4 of TC Earthquake, Working Group 1 of TC Concrete (EC2) and participated in various conferences, seminars and workshops on Eurocodes.

IEM DIARY OF EVENTS

Technical Talk: The Niched Role of Traction Power System in the Electrical Power Engineering Profession for Malaysia's Electric-based Railway Public Transportation Infrastructure Projects

30th September 2014

: Electrical Engineering Technical Division
: 5.30 p.m. – 7.30 p.m.
: Wisma IEM
: 2

Talk on Fixing of Fees by Professional Bodies under the Competition Act 2010 - Issues and Impact

19th September 2014

Organised by	: Standing Committee by Professional Practise
Time	: 5.30 p.m. – 7.30 p.m.
Venue	: Wisma IEM
CPD/PDP:	:2

Kindly note that the scheduled events below are subject to change. Please visit the IEM website at www.myiem.org.my for more information on the upcoming events.







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	Management (including project/contract/equipment/service/transport		Dams/reservoirs/irrigation	Cement	
	district manager, clerk of works, other technical or operating manager)		Harbours/offshore structures	Other construction materials	
	Engineering/Design (including chief engineer, chief designer, civil/		Foundations/tunnels		
	highway/mechanical/planning engineer, other engineering/design title)		Pipelines/refineries Structures/steel work	Construction equipment	
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Our esteemed readership consists of certified engineers, decision making corporate leaders, CEOs, government officials, project directors, entrepreneurs, project consultants, engineering consulting firms and companies involved with engineering products and services.

IEM DIRECTORY is circulated to more than **30,000 registered members** of The Institution of Engineers, Malaysia (IEM), with an **estimated readership of 120,000** professionals.

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Our business partners can be assured that their products and services will be given the circulation and exposure it deserves, thus maintaining a sustained advertising presence to our core readers of decisionmaking engineers and technical experts. Our website offers an even wider market reach, with added international presence, aided by our international affiliation with official engineering bodies all over the world. Our online and offline advertising features such as banner advertising, article sponsorship and direct e-mail announcements have proven to be successful marketing strategies that will set the businesses of our partners apart from their competition.

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- Malaysia's most comprehensive directory of engineering companies and consulting firms.
- Detailed descriptions of major and on-going projects.
- A decisive listing of companies providing goods and services to engineers and engineering businesses in a concise format, as well as an easily searchable index of companies sorted by engineering disciplines.
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- List of major Engineering related events, seminars and exhibitions in Malaysia.



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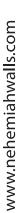
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